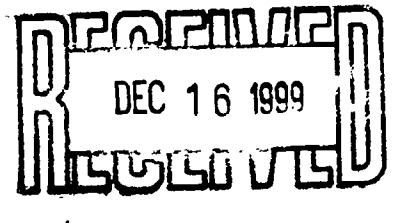


15346
 DESCRIPTION: Res
 CONTRACTOR: Bandal Coat #10734
 ADDRESS: 12 Cragmoor Village Rd
 Plot plan Sets of Plans
 Permit Number D16087 Zone _____ City _____ County _____
 APPROVED FOR CONSTRUCTION
 Date 4/19/00 Issued _____ By AN



63282-00-037
 (30 psf snow)

1876 ft = 300 sq
 per fee 75.00 pd R# 350877

	Approved	Disapproved	Approved as Corrected	By	Date
Construction	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	STL	1/6/00
Mechanical	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
Electrical	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
Plumbing	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
Health	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
Traf. Eng.	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
City Eng.	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
Zoning	<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	STL	2/30/00
Planning	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
Enumeration	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		12/16/99
Fire	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		
Flood Plain	<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>		

fc 1/1/00
 SEE REQUIRES

MINIMUM DETAIL DRAW PENDING FINALIZATION OF PLAT PER DP 00-63 SEE LIMITS OF DISTURBANCE ON SITE PLAN
 Attach Approved (Stamped) Development Plan With Resubmittal
 RECORDED
 SHOW PROOF OF LEGAL LOT OR ATTACH COPY OF RECORDED PLAT

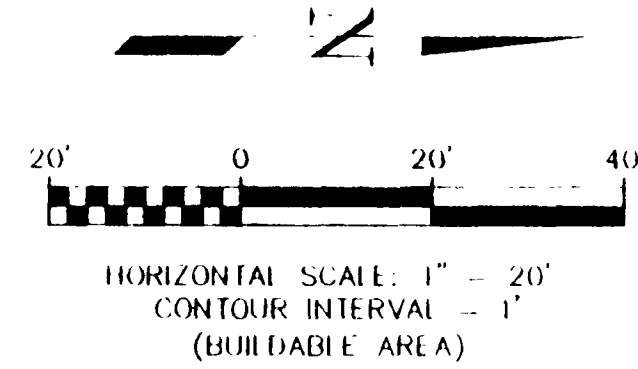
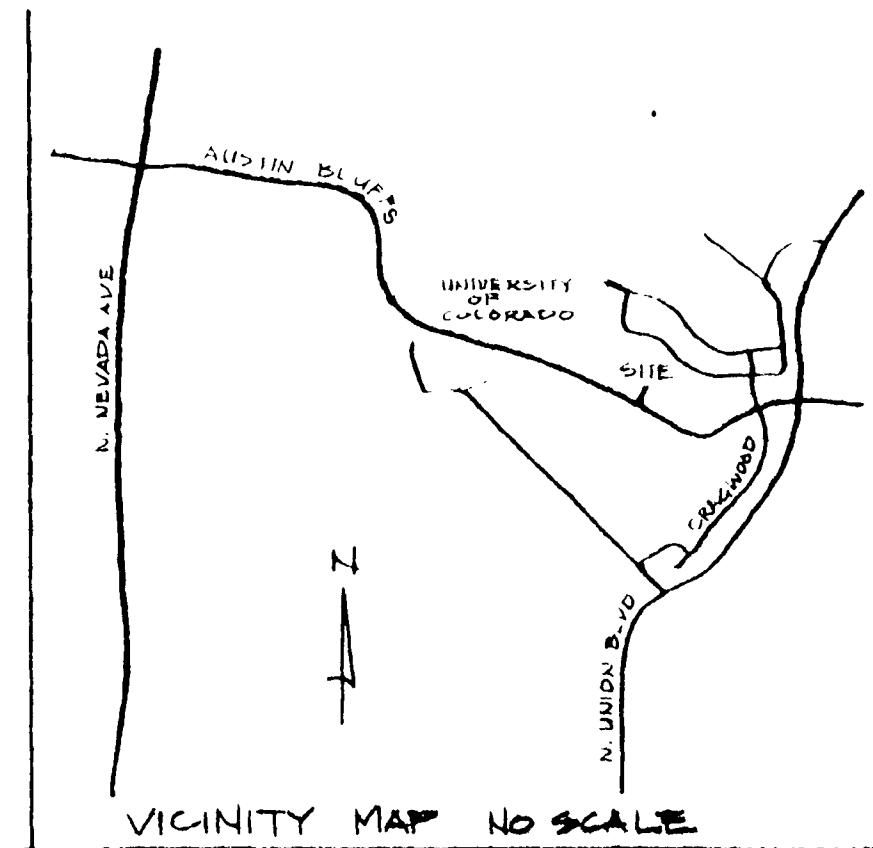
Type of Unit

<input type="checkbox"/> Ranch	Basement	1312	sq. ft.
<input type="checkbox"/> Ranch	Ground Level	1326	sq. ft.
<input type="checkbox"/> Ranch	Upper Level		sq. ft.
<input type="checkbox"/> Ranch	Upper Level 2		sq. ft.

26298

PATTERSON SUBDIVISION

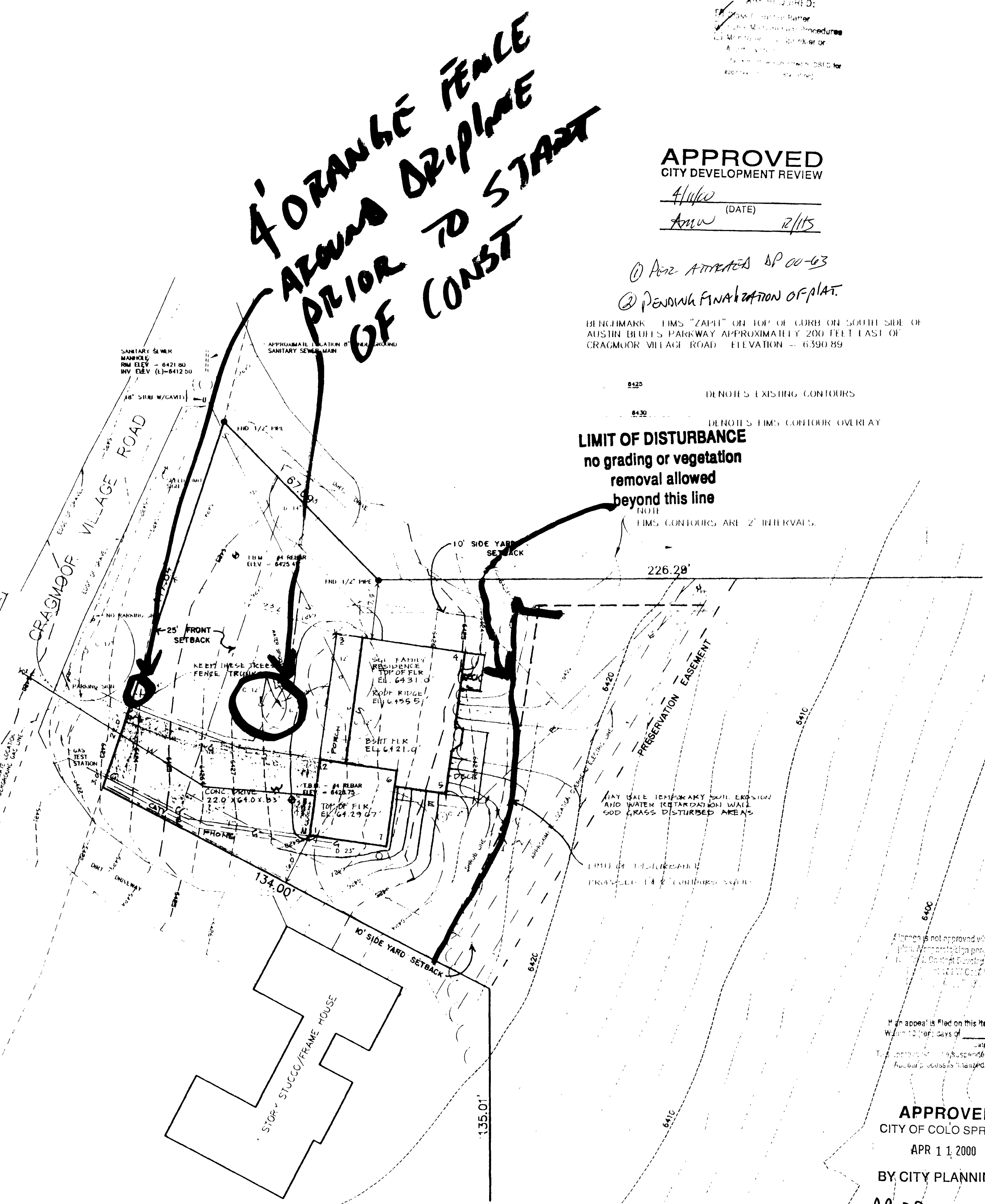
Property Owner's Name: BRIAN & EVELYN PATTERSON
 Address: 12 Cragmoor Village Road
 Telephone: 719 534 9200 OR 353 4370 BUREAU
 Applicant's name: KANPAI CONSTRUCTION, INC.
 Address: 5410 WINDLETON COURT, COLORADO SPRINGS
 Telephone: 719 599 3244, COLORADO 80918
 Land Planners Name: [Blank]
 Address: [Blank]
 Telephone: [Blank]
 Land surveyor's Name: LDC, INC.
 Address: 117 528 6133
 Telephone: [Blank]
 Site Address: CRAGMOOR VILLAGE ROAD
 Tax Schedule Number: 03282-00 037
 Zoning District Classification: R-15 (ESTATE WITH HISTORIC OVERLAY)
 Setbacks front 25' side 10' rear 30'
 Property Area: 1.424 ACRES 62,071 SQ FT
 Building Footprint: 2,469 SQ FT
 Total Lot Coverage: 3.17 %



"Hillside Protection Notes"
 Note 1: No disturbance, grading or significant natural features and vegetation removal will occur beyond the "Limit of disturbance" line as shown on this plan.
 Note 2: The "Limit of Disturbance" line shall be delineated during construction with flags, roping, and or four feet tall orange construction fencing.

LEGAL DESCRIPTION:

RELOCATE EXISTING GAS TEST STATION AND PARKING SIGN IF REQUIRED.
 EQUIPMENT MATERIAL AND SOIL STORAGE SHALL BE IN DRIVEWAY AREA.



ITEMS CHECKED BELOW
 ASSESS 2.01.01
 ASSESS 2.01.02
 ASSESS 2.01.03
 ASSESS 2.01.04
 ASSESS 2.01.05
 ASSESS 2.01.06
 ASSESS 2.01.07
 ASSESS 2.01.08
 ASSESS 2.01.09
 ASSESS 2.01.10
 ASSESS 2.01.11
 ASSESS 2.01.12
 ASSESS 2.01.13
 ASSESS 2.01.14
 ASSESS 2.01.15
 ASSESS 2.01.16
 ASSESS 2.01.17
 ASSESS 2.01.18
 ASSESS 2.01.19
 ASSESS 2.01.20

APPROVED
 CITY DEVELOPMENT REVIEW

4/16/00 (DATE)
 ARW 2/1/00

- 1) PER APPROVED DP 00-063
- 2) PENDING FINALIZATION OF PLAN

BENCHMARK: TMS "ZAP1" ON TOP OF CURB ON SOUTH SIDE OF AUSTIN BLUFFS PARKWAY APPROXIMATELY 200 FEET EAST OF CRAGMOOR VILLAGE ROAD ELEVATION = 6390.89

LIMIT OF DISTURBANCE
 no grading or vegetation removal allowed beyond this line

PROPOSED STRUCTURE SITS ON TOP OF HILL. ALL DRAINAGE IS AWAY FROM STRUCTURE ALONG EXISTING PATTERNS. NO ADVERSE EFFECTS ON NEIGHBORS LOTS.

CORNER	FIN GRADE
1	6,429.67
2	6,429.67
3	6,428.8
4	6,424.5
5	6,424.5
6	6,428.3
7	6,428.3

19.11.97
 AVERAGE FINISHED GRADE: 6,427.73'
 PLUS 36" = 6,462.73" MAX HGT

CALL BEFORE YOU DIG...
 1-800-922-1987
 LDC, Inc.
 PLANNING, SURVEYING, LAND SERVICES
 3520 Austin Bluffs Parkway
 Colorado Springs, CO 80918
 (719) 528-6133 FAX (719) 528-6848

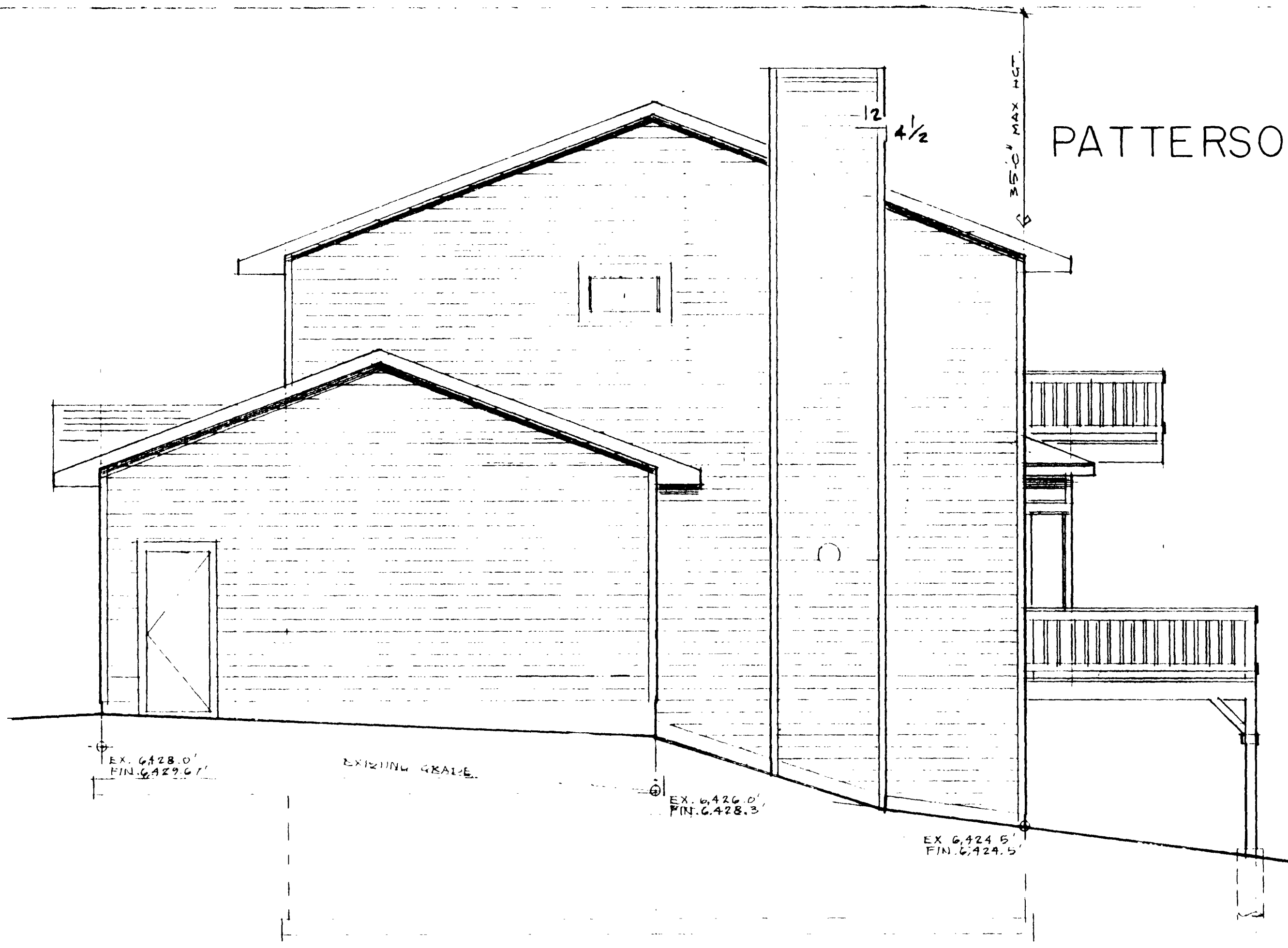
Notice: According to Colorado law you must commence any legal action based upon any defect in this survey within three years after you first discover such defect in no event may any action based upon any defect in this survey be commenced more than ten years from the date of the certification shown hereon.

NO.	REVISIONS DESCRIPTION	BY	DATE

PATTERSON SUBDIVISION
 HILLSIDE DEVELOPMENT/SITE PLAN
 PROJECT NO. 99124
 Drawn By: RDG
 Checked By: DVH
 Date: 10/11/99
 Sheet: 2 of 2

APPROVED
 CITY OF COLO SPRINGS
 APR 11, 2000
 BY CITY PLANNING
 AR DP 00-063

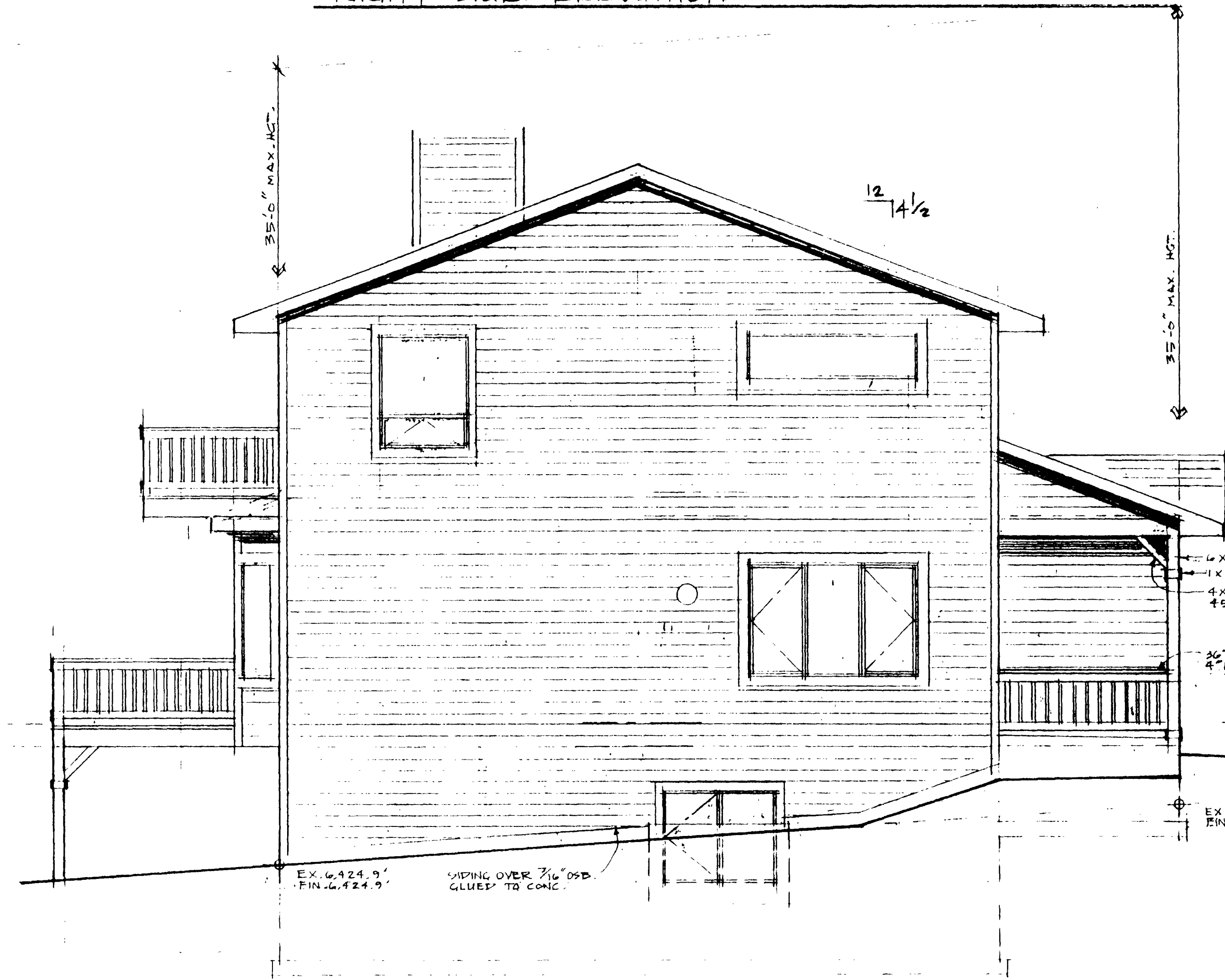
PATTERSON SUBDIVISION



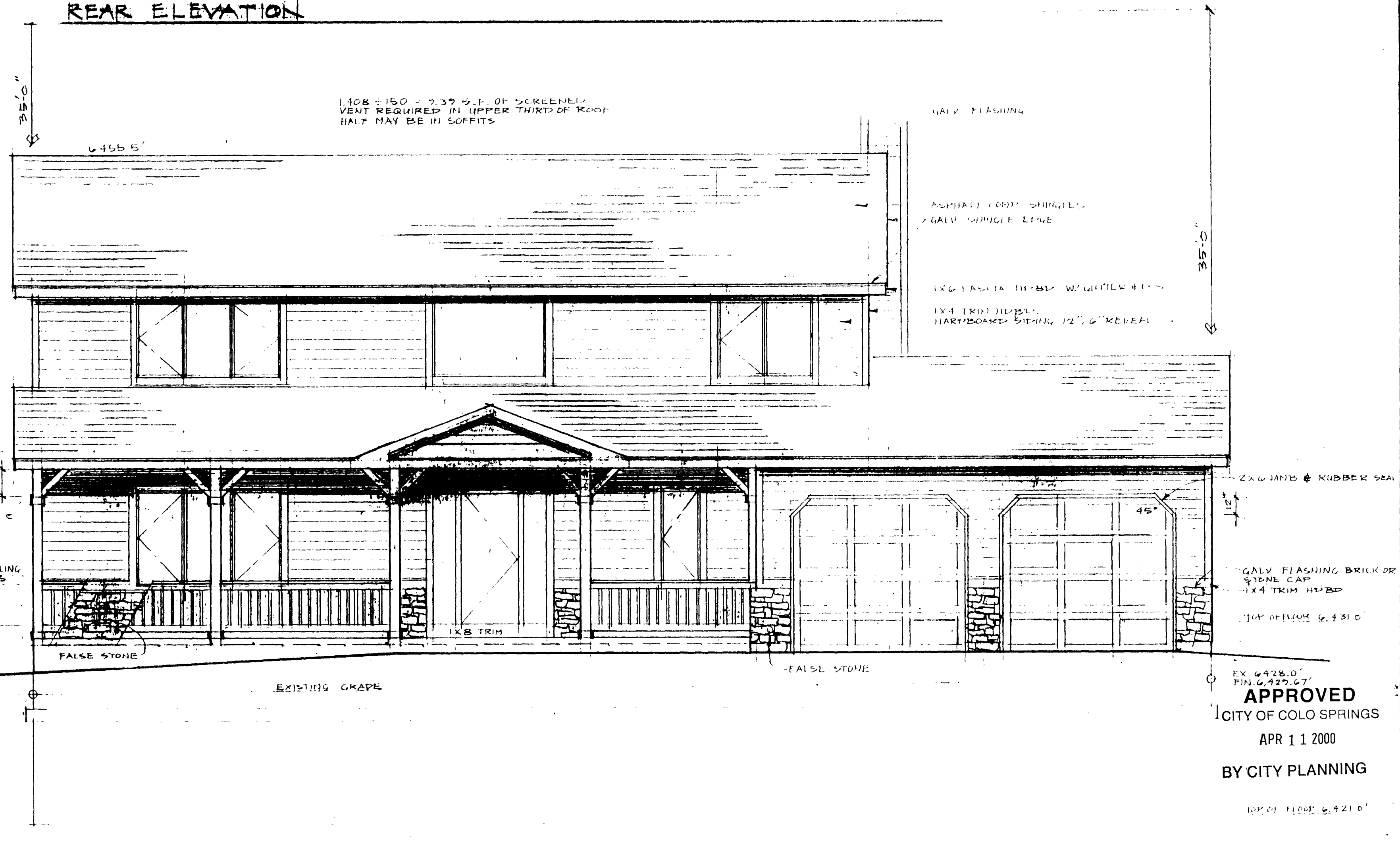
RIGHT SIDE ELEVATION



REAR ELEVATION



LEFT SIDE ELEVATION



FRONT ELEVATION

APPROVED
CITY OF COLO SPRINGS
APR 11 2000
BY CITY PLANNING

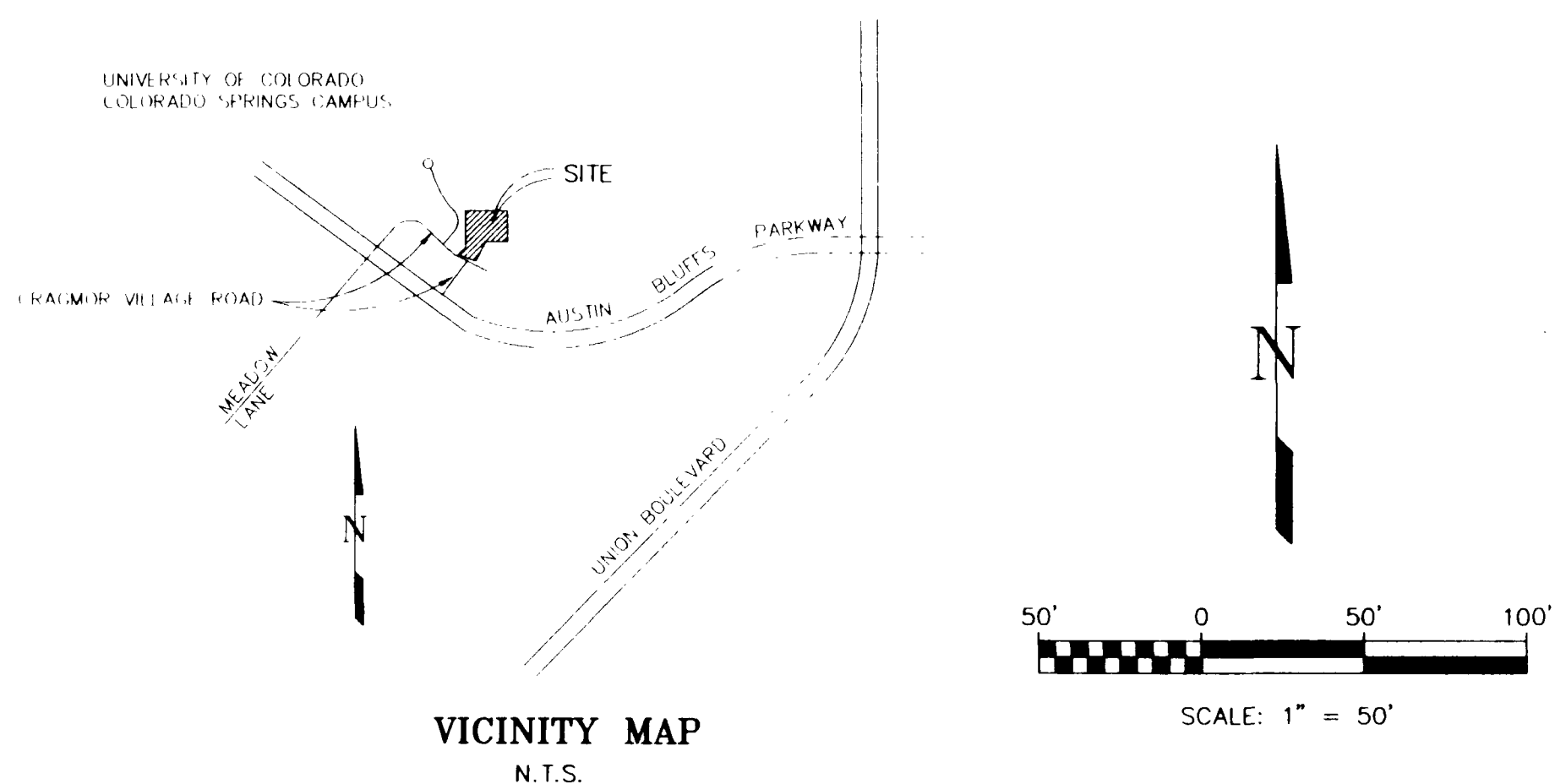
BRIAN & EVELYN PATTERSON
RANDAL CONSTRUCTION, INC.
5410 WIDEON POINT
COLORADO SPRINGS, CO. 80918
719.599.3244

DICK WOODS
214 BOB WHITE
COLORADO CITY
COLORADO 81019

SHEET
3
OF
3

NO. 20.00.01.2

PATTERSON SUBDIVISION



LEGAL DESCRIPTION

A tract of land situated in the South One-Half of the Southwest One-Quarter of the Southwest One-Quarter of the Northwest One-Quarter and of the North One-Half of the Northwest One-Quarter of the Southwest One-Quarter of Section 28, Township 13 South, Range 66 West of the 6th P.M., El Paso County, Colorado, being more particularly described as follows:

Beginning at a point which lies 330.0 feet North and 400.0 feet East of the Southwest corner of the Northwest One-Quarter of said Section 29; thence East a distance of 260.00 feet; thence South 397.00 feet; thence N64°26'00"W 155.19 feet; thence N71°22'00"W 177.84 feet; thence N45°47'00"E 67.70 feet; thence North 226.00 feet, more or less, to the Point of Beginning.

EXCEPT THAT PORTION DESCRIBED AS FOLLOWS:

A tract of land in the South One-Half of the Southwest One-Quarter of the Southwest One-Quarter of the Northwest One-Quarter and the North One-Half of the Northwest One-Quarter of the Southwest One-Quarter of Section 28, Township 13 South, Range 66 West of the 6th P.M., in the City of Colorado Springs, County of El Paso, State of Colorado, described as follows:

Commencing at a point 330 feet North and 400 feet East of the West One-Quarter corner of Section 28; thence East parallel with and 330 feet North of the South line of the Southwest One-Quarter of the Northwest One-Quarter, 260 feet to the East line of the Southwest One-Quarter of the Southwest One-Quarter of the Northwest One-Quarter; thence angle right 90° South, 192 feet to the Point of Beginning of the tract of land to be described hereby; thence continuing on the last mentioned course, 205 feet; thence angle right 115°34' Northwesterly, 155.10 feet; thence angle left 6°56' Northwesterly 60 feet; thence angle right 8°51' Northeasterly 134 feet; thence angle right 26°31' Easterly 135 feet to the Beginning, according to the Plat there, County of El Paso, State of Colorado.

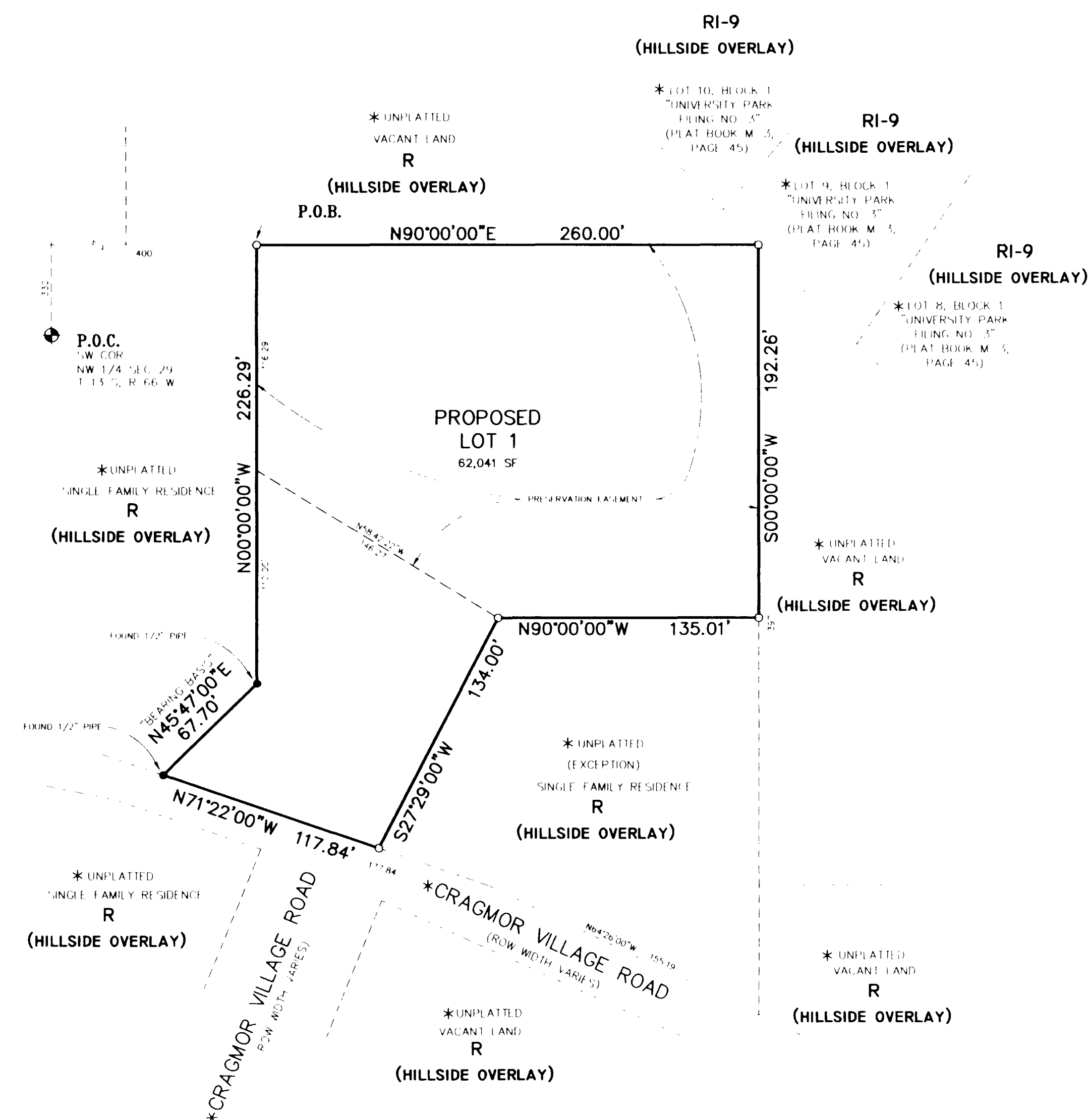
Containing 1.424 acres, more or less.

GEOLOGICE HAZARD DISCLOSURE STATEMENT

This property is subject to the findings, summary and conclusions of a Geologic Hazards Report prepared by RMG Engineers dated January 31, 2000. A copy of said report has been placed within file AR DP 00-063 of the City of Colorado Springs - Development Services Division. Contact Development Services Division, 30 South Nevada Avenue, Ste. 301, Colorado Springs, CO, if you would like to review said report.

FLOOD NOTE

FEDERAL EMERGENCY MANAGEMENT AGENCY, Flood Insurance Rate Map Number 08041C 0518 F, effective date, March 17, 1997, indicates the area in the vicinity of this parcel of land to be a Zone X (area determined to be out of the 500 year flood plain).



NOTES

- All lots within this development are subject to fuels management requirements. It is the responsibility of the builder to implement the fuels management Procedures as defined in Section 105 in part 1 of Article 4 of Chapter 20 of the City Code for each lot. Approval inspection must be obtained from the City Zoning Administration office prior to Final inspection by the Building Department and/or allowing occupancy of the residence. The initial fuels management inspection must be requested from the City Zoning Administration office prior to framing inspection with subsequent approval obtained prior to building final.
- Generally, slopes greater than twenty-five percent (25%) have been avoided by creating and including them within preservation/ no build areas and therefore excluding them from the determined building envelopes.
- 4' orange fence to be installed around drip-line of all trees to be saved prior to start of construction.
- Class C roof coverings are required.
- Residing in or near wildland interface or intermix area involves increased fire risks that may apply in urban or more urbanized types of developed communities.
- Building envelopes are the areas within a building lot not included within a required setback or preservation/no build area.
- Preservation Easement:** It shall be unlawful for any person to construct any structure, remove any live vegetation, perform any filling or grading, or to otherwise disturb within a Preservation Easement area. This provision shall not preclude the normal trimming and maintenance of vegetation material, or the planting of additional landscaping within a Preservation easement area or selective removal of vegetation required by the City to comply with the Wildfire Fuels Management provisions as listed in Section 20-4-105 of the City Code.

WILDLIFE INTERFACE NOTES

Impact Analysis:
Overall Impact: Low (Urban Designation)

Ranges:
Mule Deer: Overall District - Overall Winter Range
Morning Dove: Overall District
(Interface Data provided by Colorado Department of Wildlife Risk Maps)

Ecosystem:
Transitional, Foothills Shrublands
No apparent nesting.
No areas of standing or flowing water.

Buildable Area:
8% to 15% slope, sandy, vegetated with dryland grasses, yucca, cactus, wildflowers, two douglas firs, two limber pines, two deciduous trees, and some gambles oak.

Preservation Area:
Moderate to steep slopes, dense stands of gambles oak.

Significant Natural Features:
Moderate to steep slopes with significantly dense stands of gambles oak.

Significant Vegetation Removal
Two limber pines at the front of the proposed dwelling.
One deciduous tree at the corner of the proposed garage.
Some gambles oak will be trimmed at the rear of the dwelling and along the proposed electric service.

APPROVED
CITY OF COLO SPRINGS
APR 11 2000
BY CITY PLANNING

CITY FILE NO. AR DP 00-63

LDC, Inc.
PLANNING, SURVEYING, LAND SERVICES
3520 Austin Bluffs Parkway
Colorado Springs, CO 80918
(719) 528-6133 FAX (719) 528-6848

REVISIONS			
NO.	DESCRIPTION	BY	DATE
1	ADDED PRESERVATION EASEMENT	RDG	01/03/00
2	ADDED NOTES	KAC	3-21-00

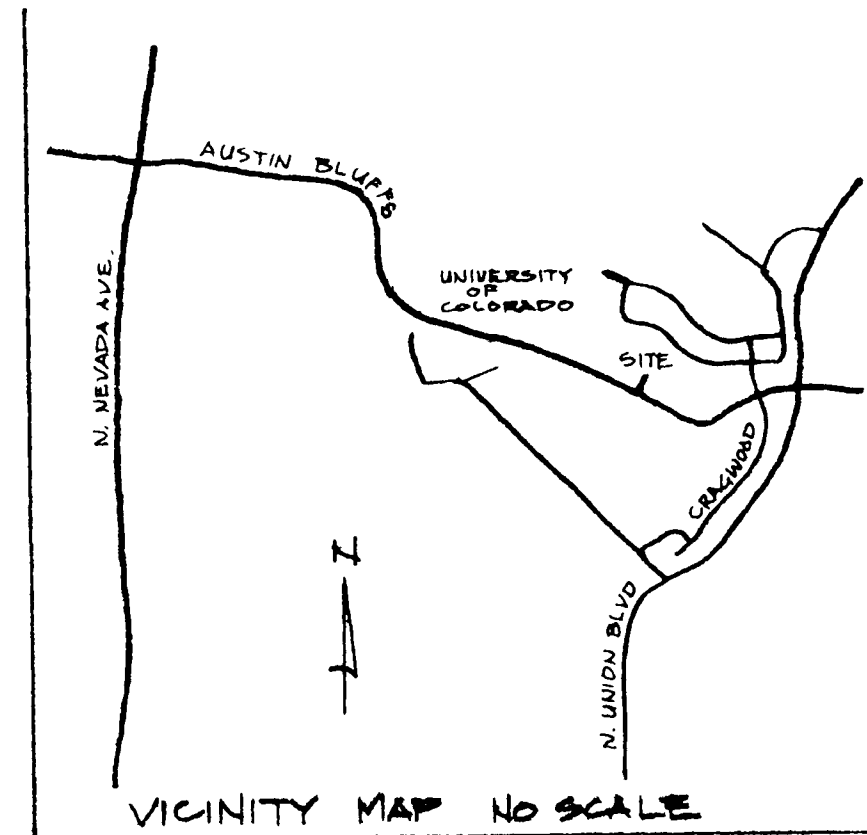
Notice: According to Colorado law you must commence any legal action based upon any defect in this survey within three years after you first discover such defect. In no event may any action based upon any defect in this survey be commenced more than ten years from the date of the certification shown herein.

PROJECT NO. 99124		Drawn By: AJB	Date: 11/05/99
		Checked By: DVH	Sheet: 1 of 1



F:\99000\99124 - Cragmor Village Road Property\99124p1.dwg, Tue Apr 04 15:55:42 2000

Property Owner's Name: BRIAN & EVELYN PATTERSON
Address: 12 CRAGMOOR VILLAGE ROAD
Telephone: 719-534-9240 OR 333-2370 80918
Applicant's name: RANDAL CONSTRUCTION, INC.
Address: 5410 WILDEN POINT COLORADO SPRINGS
Telephone: 719-599-3244 COLORADO 80918
Land Planners Name:
Address:
Telephone:
Land surveyor's Name: LDC, INC.
Address:
Telephone: 719-528-6133
Site Address: CRAGMOOR VILLAGE ROAD
Tax Schedule Number: 63282-00-037
Zoning District Classification: R-ESTATE WITH HILLSIDE OVERLAY
Setbacks: front 25' side 10' rear 30'
Property Area: 1.424 ACRES 62,041 S.F.
Building Footprint: 2,397 S.F.
Total Lot Coverage: 3.86%



"Hillside Protection Notes"
 Note 1. No disturbance, grading or significant natural features and vegetation removal will occur beyond the "Limit of Disturbance" line as shown on this plan.
 Note 2. The "Limit of Disturbance" line shall be delineated during construction with flags, roping, and or four feet tall orange construction fencing.

LEGAL DESCRIPTION:

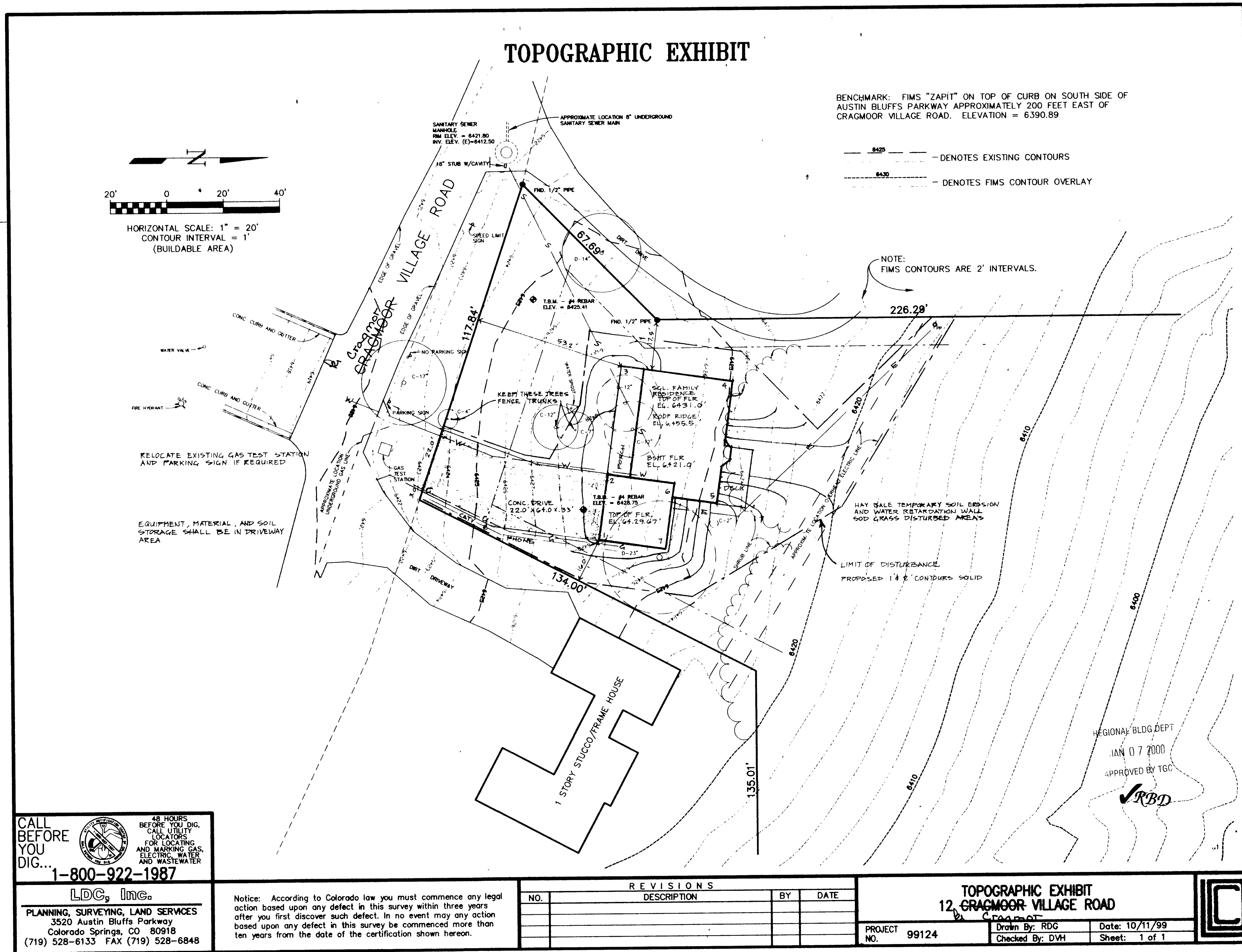
S2 SW4 SW4 NW4
 N2 NW4 NW4 SW4
 Sec. 28-13-66

PROPOSED STRUCTURE SITS ON TOP OF HILL. ALL DRAINAGE IS AWAY FROM STRUCTURE ALONG EXISTING PATTERNS. NO ADVERSE EFFECTS ON NEIGHBORS' LOTS.

CORNER	FIN. GRADE
1	6,427.67
2	6,427.67
3	6,428.8
4	6,424.9
5	6,424.5
6	6,428.3
7	6,428.3

AVERAGE FINISHED GRADE 6,427.73'
PLUS 35' = 6,462.73' MAX. HGT.

D:\9900\99124 - Cragmoor Village Road Property Composite.dwg Thu Oct 14 11:59:59 1999



GENERAL

- The owner or builder shall be responsible for and verify before construction; building location, site conditions, dimensions, materials, colors, quantity, finishes, engineering, architects, fees, and surveyors required to participate.
 - All work shall be in accordance with current governing building codes, laws, and manufacturer's specifications.
 - Building designer's liability regarding, errors, omissions, workmanship, and construction shall be limited to the correction of the purchaser's original drawings for a period not to exceed five months after completion of final plans or additional compensation may be required by designer. Do not scale blue-line copies.
 - Dimensions are to the face of studs or concrete walls, centerline of posts and beams unless otherwise noted.
- FOUNDATION**
- Foundation design and soil report shall be at site at time of inspection and must be by a Colorado licensed engineer.
 - Default soil bearing capacity of 2,000 psf used for foundation design if no engineer is required.
 - All concrete shall be type 1, 3,000 psi minimum at 28 days. All reinforcement steel shall be grade 40, lapped 30 bar diameters minimum unless otherwise specified by engineer.
 - Provide 4 in. diameter peripheral footer duct to sump pump or daylight end at 1/4 in. per foot per engineer's specific.
 - Foundation shall be anchored to footer and shall be waterproofed where exposed to soil and water.
 - Provide 2x4 min. treated or redwood sill w/ 1/2 in. dia. by 10 in. anchor bolts at 72 in. o.c. max. 12 in. from corners.
 - Water tight expansion joints or perimeter of slabs and floor penetrations, contraction joints at 20 feet max. each way.
 - Owner shall be responsible for radon gas prevention.
 - Slope finished grade down and away from foundation a minimum of 6' in the first 10 feet and be 6 in. below siding.
- FRAMING**
- Dimensional lumber shall be hem fir #2 kiln dried #1, #1, #2, #3, #4, #5, #6, #7, #8, #9, #10, #11, #12, #13, #14, #15, #16, #17, #18, #19, #20, #21, #22, #23, #24, #25, #26, #27, #28, #29, #30, #31, #32, #33, #34, #35, #36, #37, #38, #39, #40, #41, #42, #43, #44, #45, #46, #47, #48, #49, #50, #51, #52, #53, #54, #55, #56, #57, #58, #59, #60, #61, #62, #63, #64, #65, #66, #67, #68, #69, #70, #71, #72, #73, #74, #75, #76, #77, #78, #79, #80, #81, #82, #83, #84, #85, #86, #87, #88, #89, #90, #91, #92, #93, #94, #95, #96, #97, #98, #99, #100.
 - Micro lam or versus lam beams are #1, #2, #3, #4, #5, #6, #7, #8, #9, #10, #11, #12, #13, #14, #15, #16, #17, #18, #19, #20, #21, #22, #23, #24, #25, #26, #27, #28, #29, #30, #31, #32, #33, #34, #35, #36, #37, #38, #39, #40, #41, #42, #43, #44, #45, #46, #47, #48, #49, #50, #51, #52, #53, #54, #55, #56, #57, #58, #59, #60, #61, #62, #63, #64, #65, #66, #67, #68, #69, #70, #71, #72, #73, #74, #75, #76, #77, #78, #79, #80, #81, #82, #83, #84, #85, #86, #87, #88, #89, #90, #91, #92, #93, #94, #95, #96, #97, #98, #99, #100.
 - Gypsum beams are minimum of #1, #2, #3, #4, #5, #6, #7, #8, #9, #10, #11, #12, #13, #14, #15, #16, #17, #18, #19, #20, #21, #22, #23, #24, #25, #26, #27, #28, #29, #30, #31, #32, #33, #34, #35, #36, #37, #38, #39, #40, #41, #42, #43, #44, #45, #46, #47, #48, #49, #50, #51, #52, #53, #54, #55, #56, #57, #58, #59, #60, #61, #62, #63, #64, #65, #66, #67, #68, #69, #70, #71, #72, #73, #74, #75, #76, #77, #78, #79, #80, #81, #82, #83, #84, #85, #86, #87, #88, #89, #90, #91, #92, #93, #94, #95, #96, #97, #98, #99, #100.
 - Exterior walls shall be 2x6 studs at 16 in. o.c. under most loads. 2x6 studs at 16 in. o.c. where carrying floor and roof. 12 ft. max. height. R-13 insulation. If 2x6 studs then 16 in. o.c. R-13 insulation. Interior walls shall be 2x4 studs at 16 in. o.c. with double top plate, single stud plate and triple corner studs unless noted.
 - All headers and beams over load bearing openings shall be 3-2x6 with spacers, insulators, and 2-2x6 cylinders minimum under each end unless noted.
 - All exterior and interior load bearing walls shall be provided with solid or diagonal bracing at all corners and every 24 feet maximum.
 - Floor loads to live, 10 lb. dead. Deck loads to live, 5 lb. dead. Wind loads 20 psf. live per U.B.C. sec. 1609.10.1.5. 10/15/97 edition, exposure c.
 - Typically walls are shown with 1/2 in. gypsum wall board: tops, flats, texture, and paint. Square or rounded corner board, paint and texture to be chosen by owner.

Construction Notes

Mechanical

- Liquid propane gas fired appliances are not permitted in basement or crawl space in El Paso County. Teller County requires ventilation per U.M.C. and a shutoff alarm system.
 - Provide exterior combustion air to fire chamber of gas, wood, or coal fired appliances 4" minimum diameter with a tight closing damper.
 - Vent clothes dryer, exhaust fans, and heating appliances, to exterior.
 - Provide floor drains at clothes washer and water heater.
 - Field verify telephone, television, stereo speakers, intercom, central vacuum, and electrical locations.
 - Owner or builder shall provide mechanical system design.
 - Plumber and heater shall coordinate requirements with framer. Do not cut or notch beams and joists.
 - Hardware of smoke detectors to sound at master bedroom area. Provide battery backup.
- STEEL**
- Structural steel shall be ASTM A36 minimum, and pipe columns shall be ASTM A53 (grade B) or A59.
 - Metal connectors shall be Simpson strong-tie or equal.
- ROOF**
- Owner or builder shall be responsible for obtaining roof truss engineering, framing plans and details.
 - Roof ventilation shall be 1 sq. ft. of screened vent per 300 sq. ft. of roof and bottom of roof or 1 sq. ft. per 300 sq. ft. of roof only.
 - Provide roof gutters and downspouts as required with runoff conducted 30 in. minimum from foundation.
 - Roof load 30 lb. live, below 2000 ft. (40 lb. live above 2000 ft.), 15 lb. min. dead load light or 25 lb. heavy roofing.
- WINDOWS**
- Glassing is double minimum with a minimum system. Optional low E or reflective coating, R-2 minimum.
 - Tinted glass required within 36 in. of doors, within 18 in. of the floor, or sidewalks, and bearing structures.
 - U.B.C. Egress sill height maximum 48 in. off finished floor.
 - Glazing window doors are shown on plans for bidding purposes. Verify manufacturer's rough openings before framing.
 - Wood joints and casing or gypsum board wrap, screens, color, materials, and operation chosen by owner.

Wood WWR = 0.002

Insulation Requirements / Data

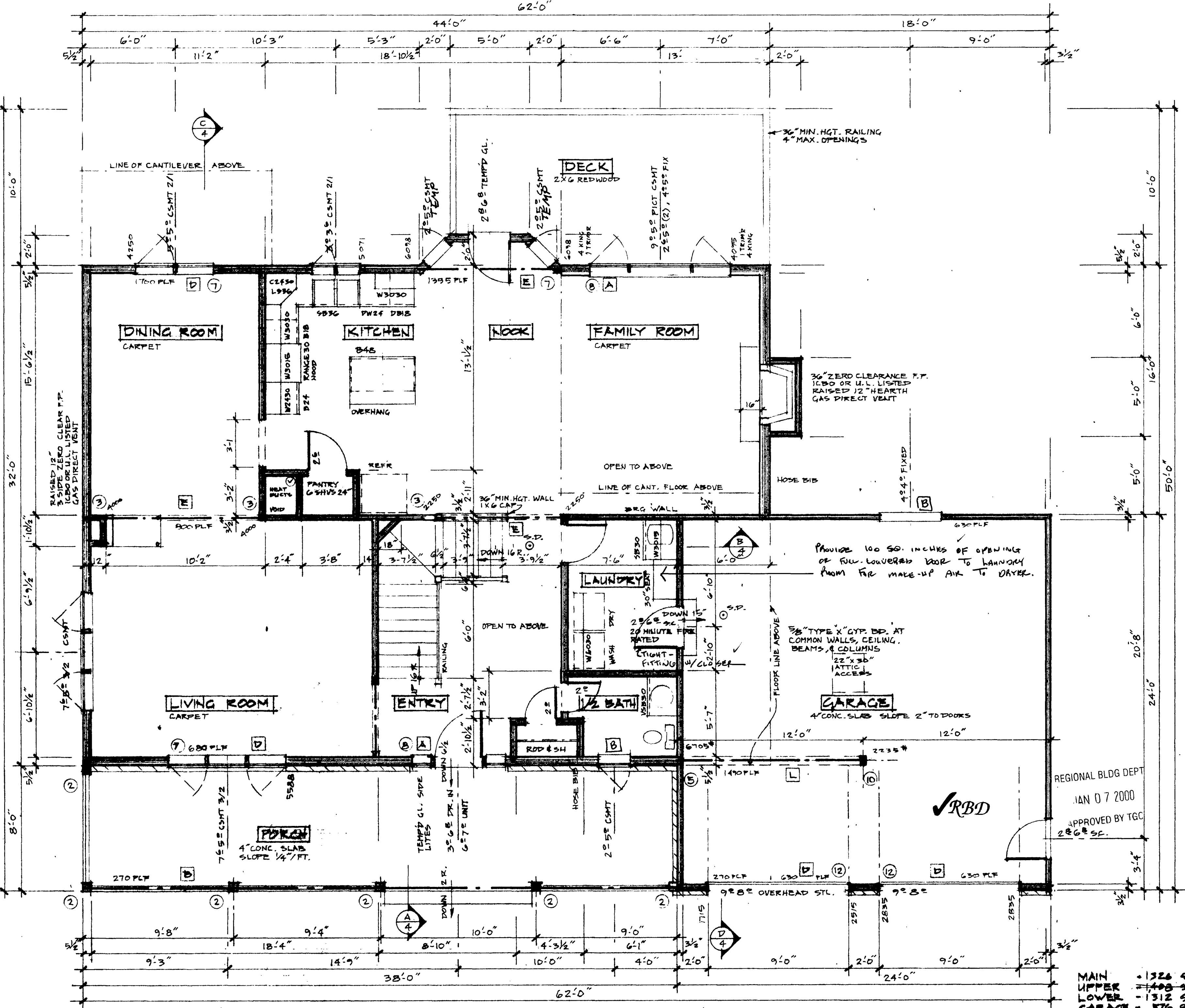
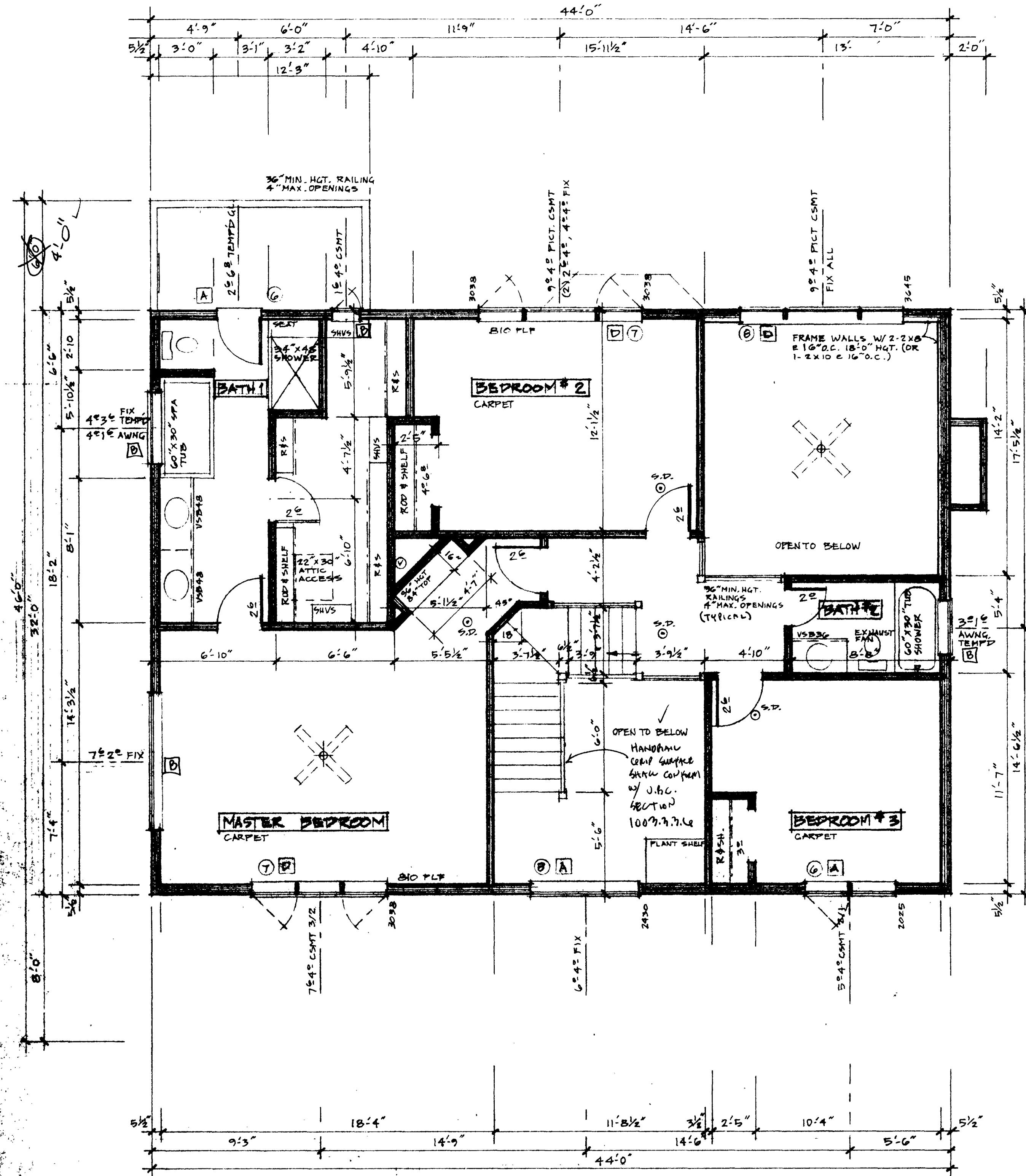
Area of Doors	132 SF.
Area of Glazing	414 SF.
Area of Wall	4,256 SF.
U Glass / window unit	0.4803
R of Wall Insulation	17
R of Roof Insulation	45
R of basement Insulation	6 MIN.
R of Crawlspace Insulation	NA
R of Foundation Insulation	6 MIN.
R of Unheated floor Insulation	30

Beam Schedule

A	3-2 x 12 header w/ 1/2" plywood spacers
B	2-2 x 12 header w/ 1/2" plywood spacers
C	2-1-3/4" x 7-1/4" Versalan
D	2-1-3/4" x 9-1/2" Versalan
E	2-1-3/4" x 11-7/8" Versalan
F	2-1-3/4" x 14" Versalan
G	2-1-3/4" x 16" Versalan
H	3-1-3/4" x 11-7/8" Versalan
J	W8 x 10# steel beam 3.94" x 7.89"
K	W8 x 13# steel beam 4.0" x 7.99"
L	W8 x 15# steel beam 4.0" x 8.11"
N	W10 x 22# steel beam 5.75" x 10.17"
O	PDL 11 1/2" BCI 45 JOIST FLUSH

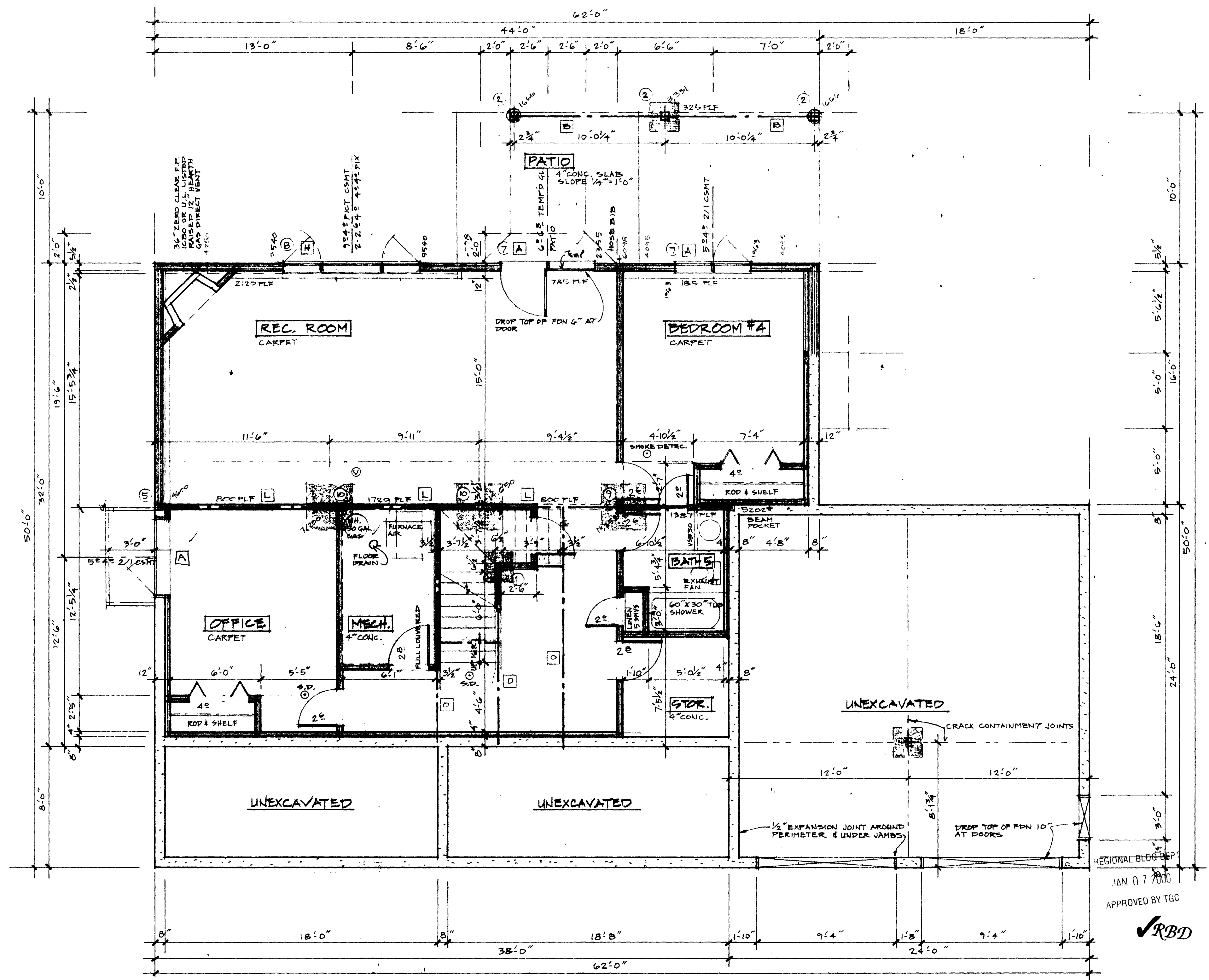
Column Schedule

1	4 x 4 post
2	6 x 6 post
3	3-2 x 4 studs
4	4-2 x 4 studs
5	3-2 x 6 studs
6	1-2 x 6 trimmer w/ 1-2 x 6 king stud
7	2-2 x 6 trimmer w/ 1-2 x 6 king stud
8	3-2 x 6 trimmer w/ 2-2 x 6 king stud
9	3" I.D. std. adj. pipe col.
10	3/4" x 3/16" steel tube
11	3-2 x 4 trimmer w/ 3-2 x 4 king stud
12	2-2 x 4 trimmer w/ 1-2 x 4 king stud



BRIAN & EVELYN PATTERSON
 12 CRACMOOR VILLAGE ROAD
RANDAL CONSTRUCTION, INC.
 2410 WIDGEDON POINT
 COLORADO SPRINGS, CO. 80918
 719.599.3244
 DRAWING BY:
DICK WOODS
 1-800-878-6858
 P.O. BOX 19135
 COLORADO CITY
 COLORADO 81013
 SHEET
 1
 NOV. 1, 1998

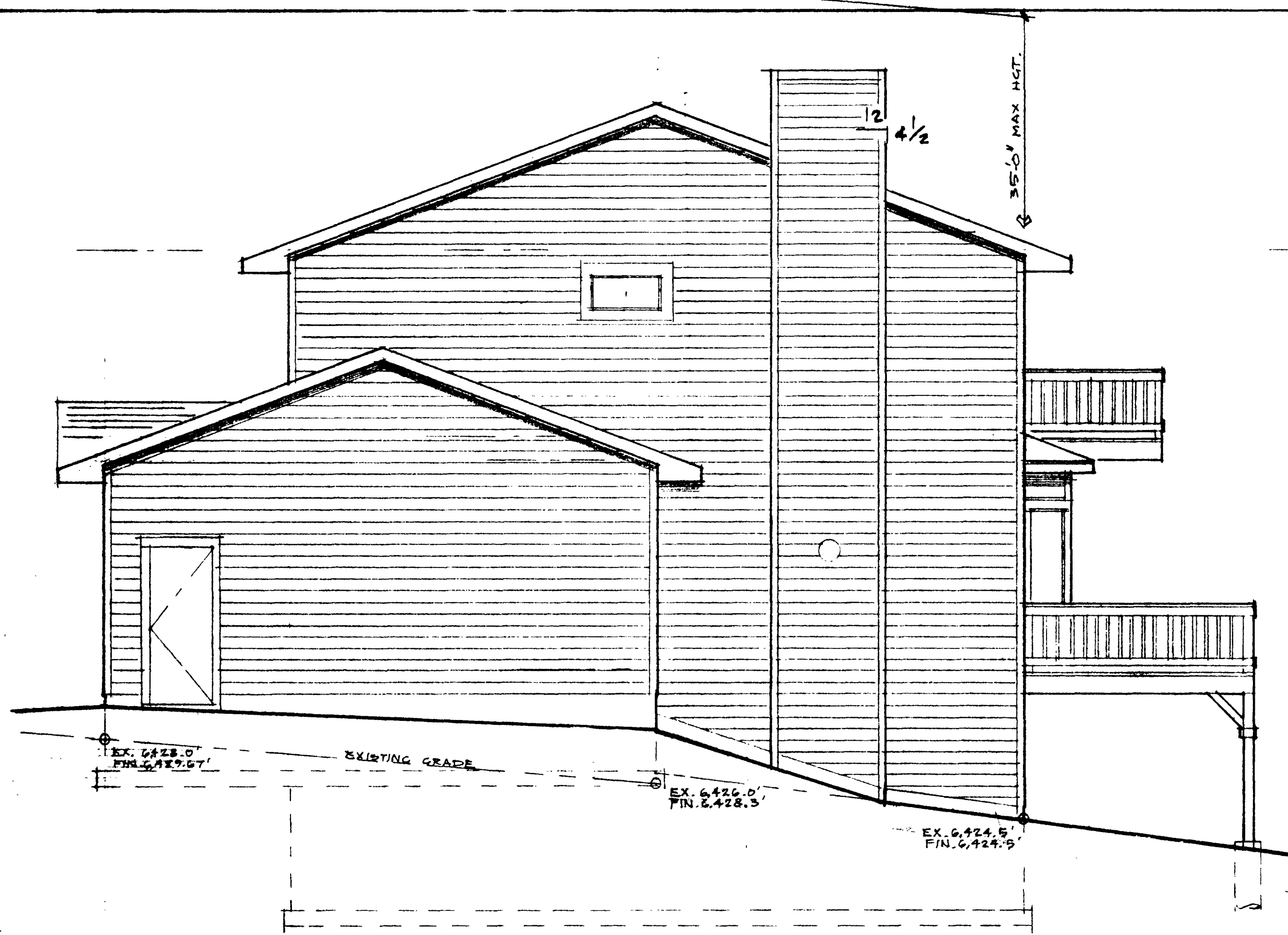
REGIONAL BLDG DEPT
 JAN 07 2000
 APPROVED BY TGC
 2868 SF.
 MAIN - 1326 SF.
 UPPER - 1408 SF.
 LOWER - 1312 SF.
 GARAGE - 576 SF.



LOWER LEVEL FLOOR PLAN

REGIONAL BLDG DEPT
 IAN 07 2000
 APPROVED BY TGC
 RBD

DRAWING BY: DICK WOODS
 1-800-878-6888
 P.O. BOX 19135
 COLORADO CITY
 COLORADO 81017
 RANDAL CONSTRUCTION, INC.
 BRIAN & EVELYN PATTERSON
 5410 WIDEON TRAIL
 COLORADO SPRINGS, CO. 80918
 719-599-3244
 SHEET
 2
 4
 2
 13



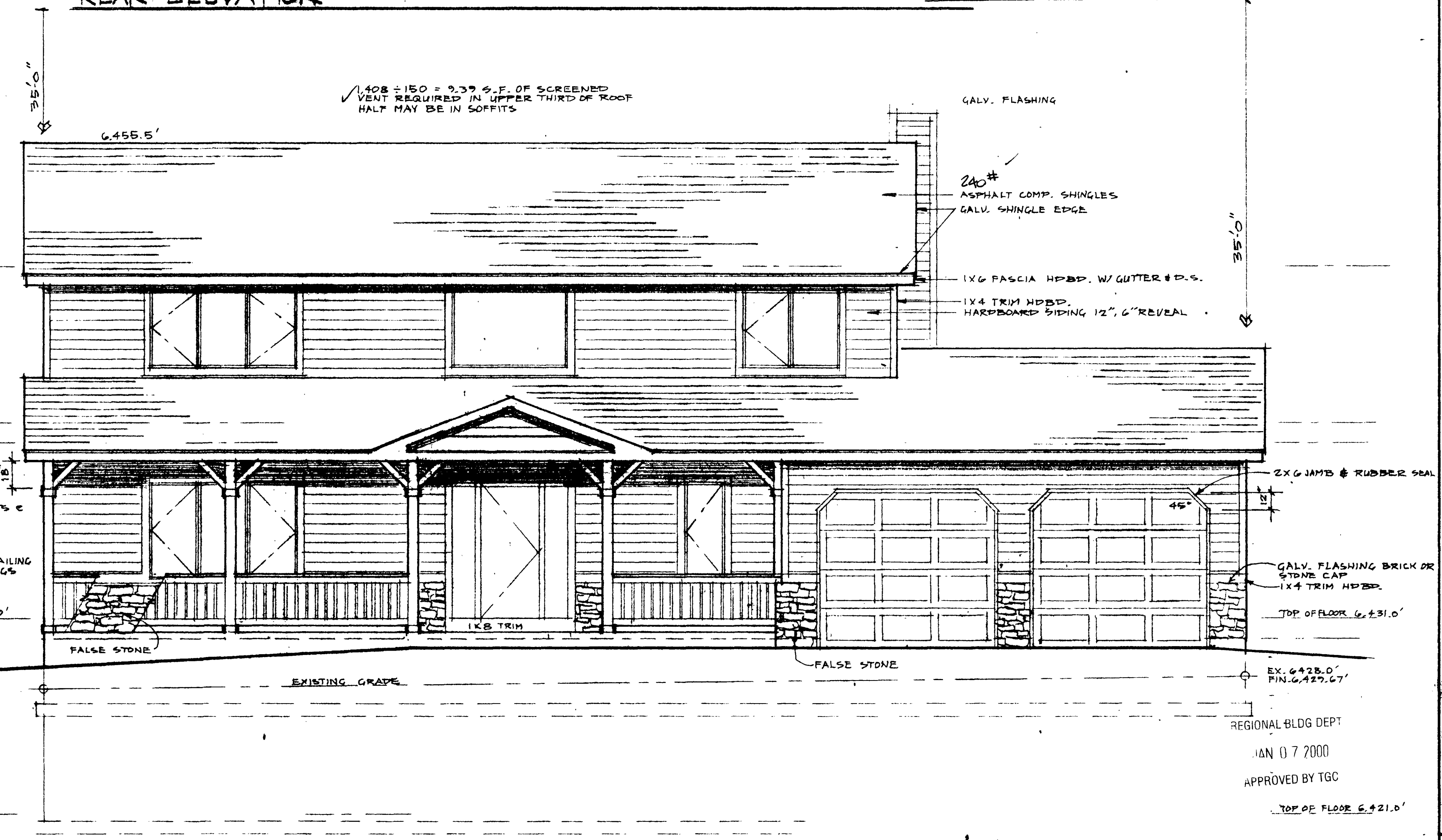
RIGHT SIDE ELEVATION



REAR ELEVATION



LEFT SIDE ELEVATION



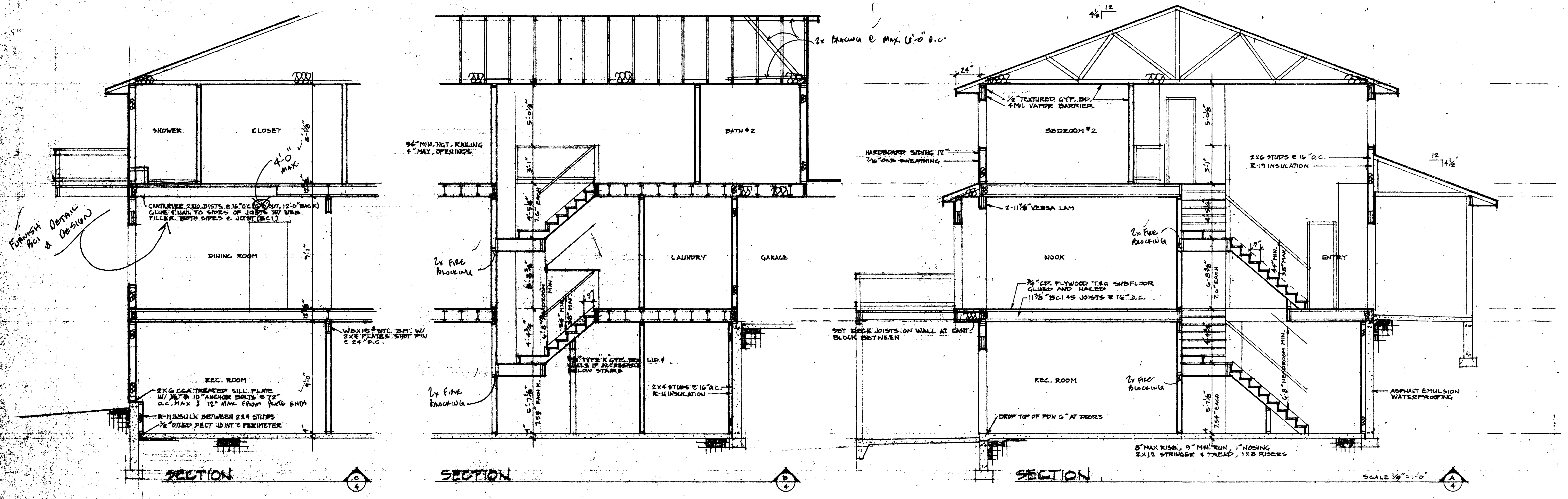
FRONT ELEVATION

REGIONAL BLDG DEPT
 JAN 07 2000
 APPROVED BY TGC
 TOP OF FLOOR 6,421.0'



RANDAL CONSTRUCTION, INC.
 5410 WIDEON POINT
 COLORADO SPRINGS, CO. 80915
 719-599-3244
 DRAFTING BY:
 DICK WOODS
 1-2-2000-03-15
 1-2-2000-03-15
 CALIFORNIA CITY
 CALIFORNIA 91311

BRIAN & EVELYN PATTERSON
 NOV 1 2000



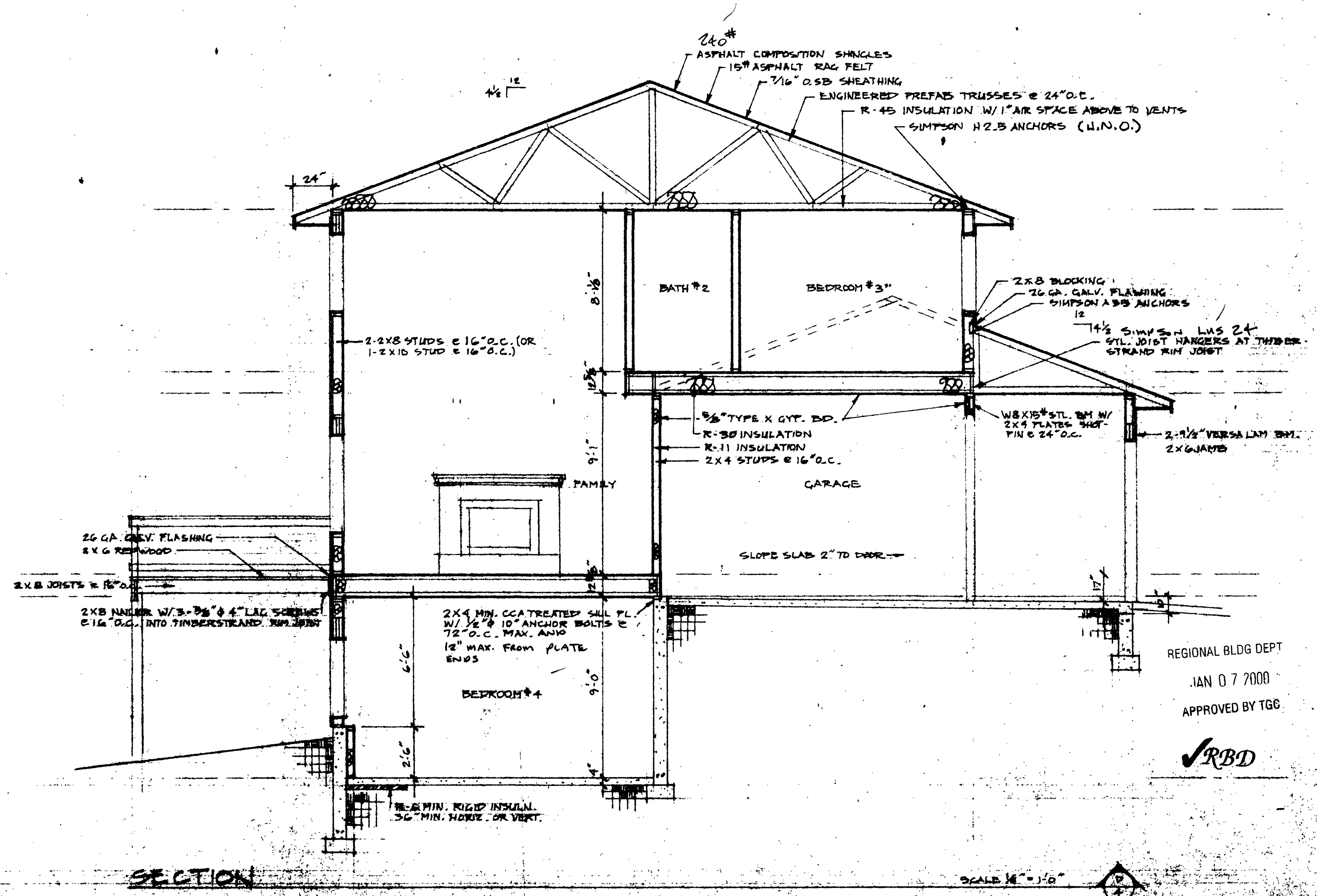
FINISH DETAIL
AS SHOWN

SECTION

SECTION

SECTION

SCALE 1/4" = 1'-0"



SECTION

SCALE 1/4" = 1'-0"

REGIONAL BLDG DEPT
JAN 07 2000
APPROVED BY TGG



KANDAL CONSTRUCTION, INC.
 BRIAN & EVELYN PATTERSON
 1000 W. WYOMING ST. #100
 COLORADO SPRINGS, CO. 80906
 TEL: 537-5121
 FAX: 537-5122

DRAWN BY
 DICK WOODS
 PROJECT NO. 001
 1000 W. WYOMING ST. #100
 COLORADO SPRINGS, CO. 80906
 TEL: 537-5121
 FAX: 537-5122

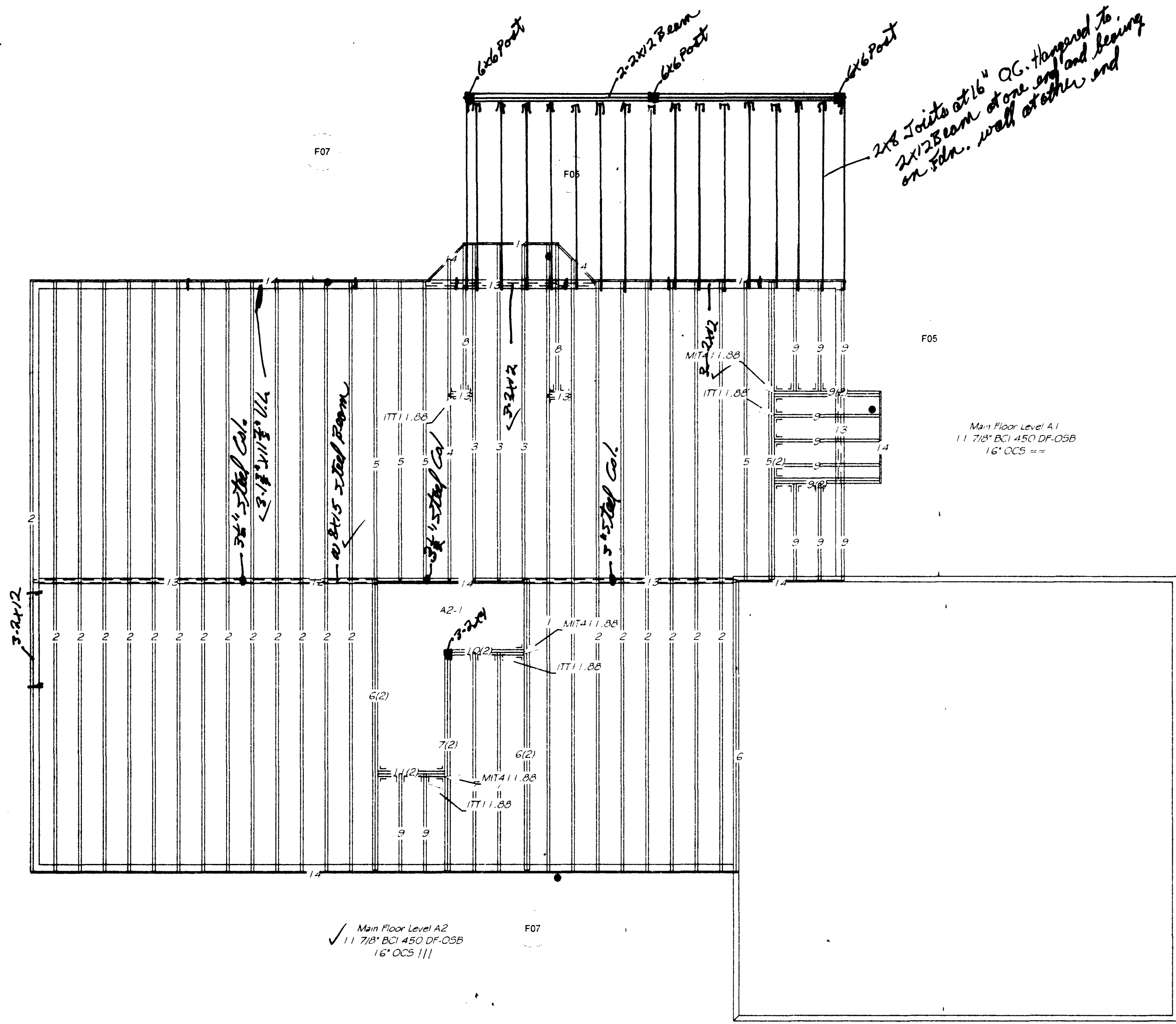
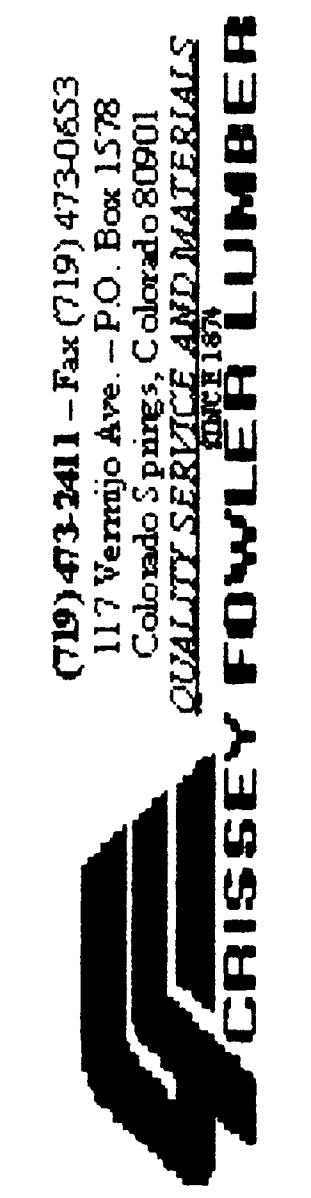
REVISIONS: BY:

Framing Schedule - Nominalized							
Mark	Qty	Description	Length	Mark	Qty	Description	Length
1	2	11 7/8" BCI 450 DF-OSB	34' 0"	8	2	11 7/8" BCI 450 DF-OSB	8' 0"
2	20	11 7/8" BCI 450 DF-OSB	32' 0"	9	15	11 7/8" BCI 450 DF-OSB	6' 0"
3	3	11 7/8" BCI 450 DF-OSB	19' 0"	10	2	11 7/8" BCI 450 DF-OSB	5' 0"
4	1	11 7/8" BCI 450 DF-OSB	18' 0"	11	2	11 7/8" BCI 450 DF-OSB	4' 0"
5	6	11 7/8" BCI 450 DF-OSB	17' 0"	12	1	STEEL BEAM	38' 0"
6	5	11 7/8" BCI 450 DF-OSB	16' 0"	13	BLK	11 7/8" BCI 450 DF-OSB	46' 8-7/8"
7	4	11 7/8" BCI 450 DF-OSB	12' 0"	14	TLF	11 7/8" Versa-Rim 98	106' 0"

Accessory Schedule			
Qty	Manufacturer	Product	Description
17	Simpson Strong-Tie Inc.	ITT11.88	1-3/4 x 11-7/8 BCI® Top Flange
5	Simpson Strong-Tie Inc.	MIT411.88	3-1/2 x 11-7/8 BCI® Top Flange



NOTE:
ALL MEASUREMENTS
TO BE VERIFIED
IN THE FIELD.



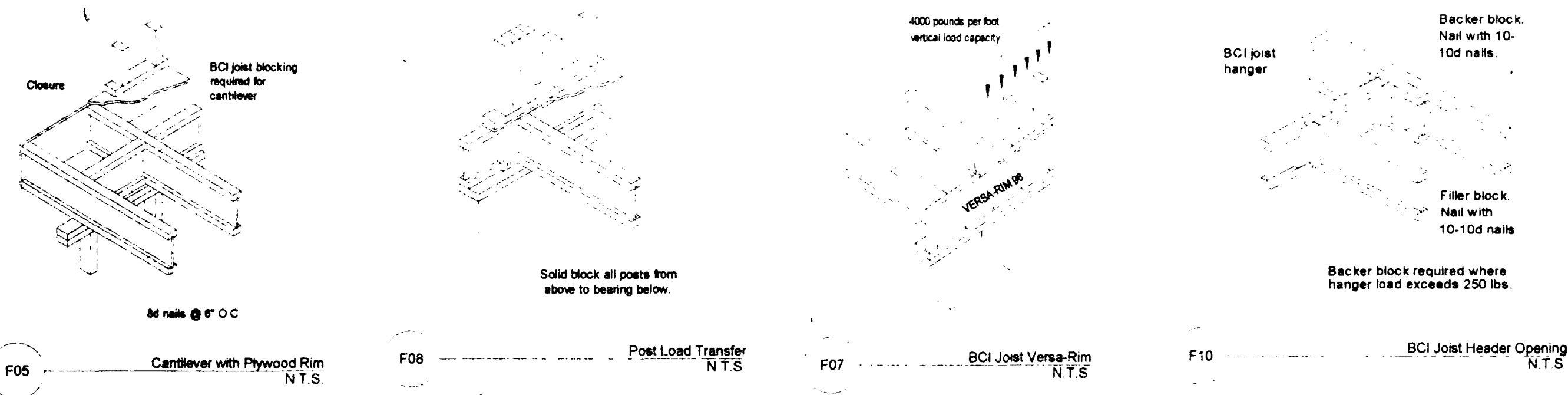
Plan View
1/4" = 1'-0"

- NOTES:
- 1) FOR ESTIMATE ONLY. USE DESIGNERS LAY-OUT FOR PLACEMENT AND NOTES.
 - 2) PLACEMENT PLAN DOES NOT CONSIDER ANY ROOF LOADS. PLEASE VERIFY ROOF LOADING WITH ROOF TRUSS MANUFACTURER.
 - 3) ALIGN FLOOR JOIST TO ALLOW FOR PLUMBING & MECHANICAL INSTALLATION.

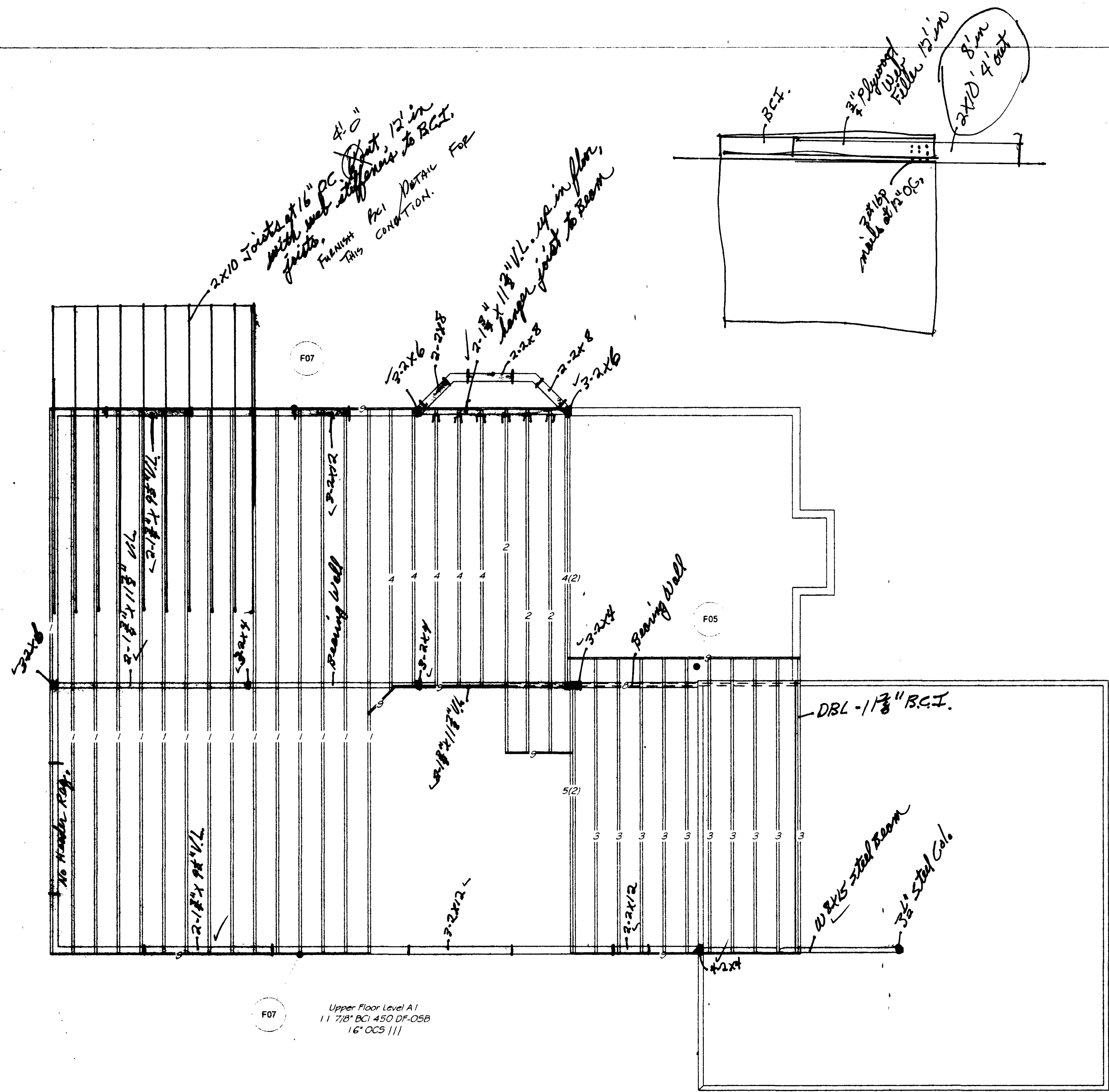
REGIONAL BLDG DEPT
JAN 07 2000
APPROVED BY TGC
JRBD

SALES PRESENTATION DRAWING
No structural or dimensional check has been made with the design drawing of the building, therefore purchaser is to check and approve all dimensions, quantities, loads and details carefully. This drawing has not been checked by Boise Cascade Engineering.

RANDAL CONSTRUCTION
PATTERSON RESIDENCE
CRISSEY FOWLER LUMBER CO.
COLORADO SPRINGS, COLORADO



SCALE: N.T.S.
DATE: 11/3/99
BY: CK
FILE: 9911611A.bcf
DWG:
SHEET: 1 / 2

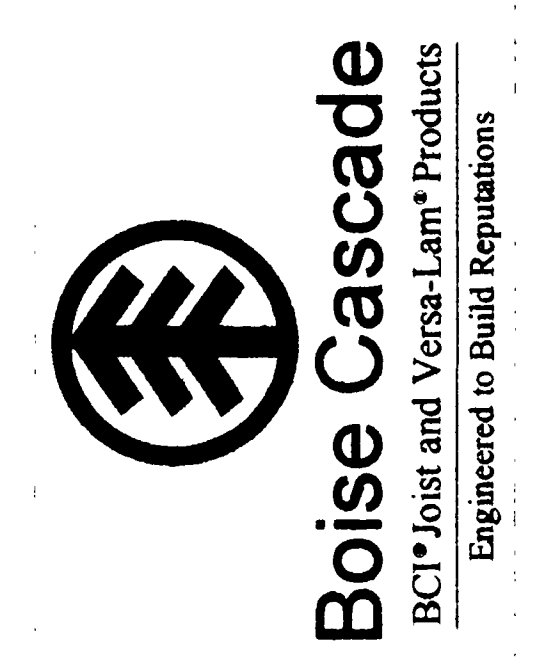


Plan View
1/4" = 1'-0"

Upper Floor Level A1
11 7/8" BCI 450 DF-OSB
16' OCS |||

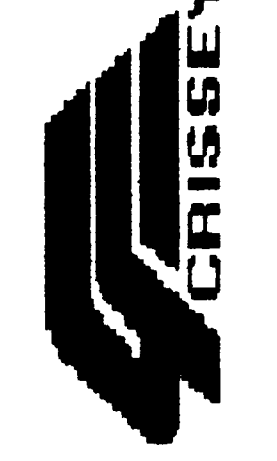
Framing Schedule - Nominalized							
Mark	Qty	Description	Length	Mark	Qty	Description	Length
1	15	11 7/8" BCI 450 DF-OSB	32' 0"	6	1	STEEL BEAM	12' 0"
2	3	11 7/8" BCI 450 DF-OSB	21' 0"	7	1	BEAM BELOW	9' 0"
3	10	11 7/8" BCI 450 DF-OSB	18' 0"	8	BLK	11 7/8" BCI 450 DF-OSB	17' 2-1/4"
4	7	11 7/8" BCI 450 DF-OSB	17' 0"	9	TLF	11 7/8" Versa-Rim 9B	92' 0"
5	2	11 7/8" BCI 450 DF-OSB	16' 0"				

REVISIONS: BY:



NOTE:
ALL MEASUREMENTS
TO BE VERIFIED
IN THE FIELD.

(719) 473-2411 - Fax (719) 473-0653
117 Vermijo Ave. - P.O. Box 1578
Colorado Springs, Colorado 80901
QUALITY SERVICE AND MATERIALS



- NOTES:
- 1) FOR ESTIMATE ONLY. USE DESIGNERS LAY-OUT FOR PLACEMENT AND NOTES.
 - 2) PLACEMENT PLAN DOES NOT CONSIDER ANY ROOF LOADS. PLEASE VERIFY ROOF LOADING WITH ROOF TRUSS MANUFACTURER.
 - 3) ALIGN FLOOR JOIST TO ALLOW FOR PLUMBING & MECHANICAL INSTALLATION.

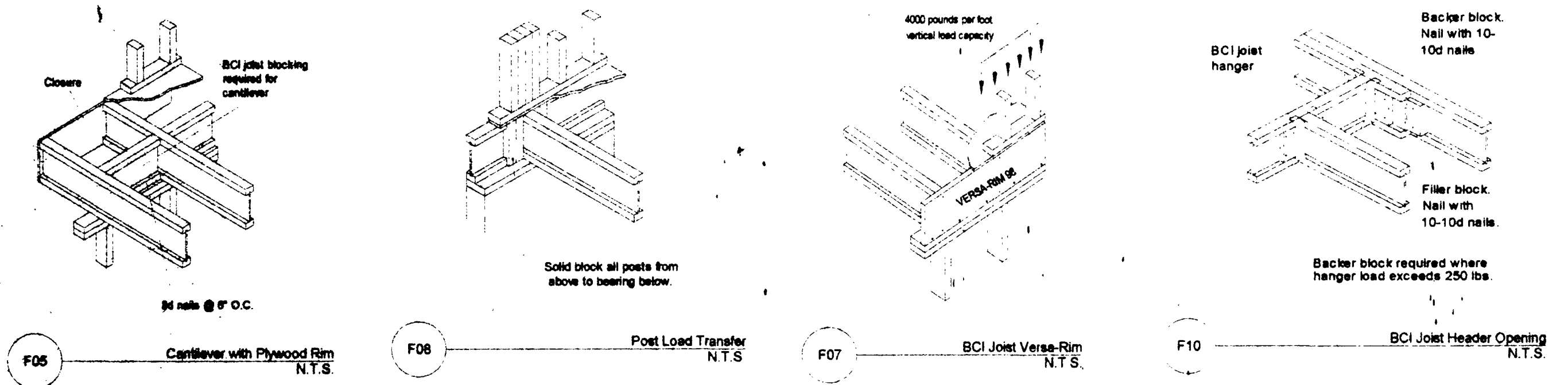
REGIONAL BLDG DEPT
JAN 07 2000
APPROVED BY TGC



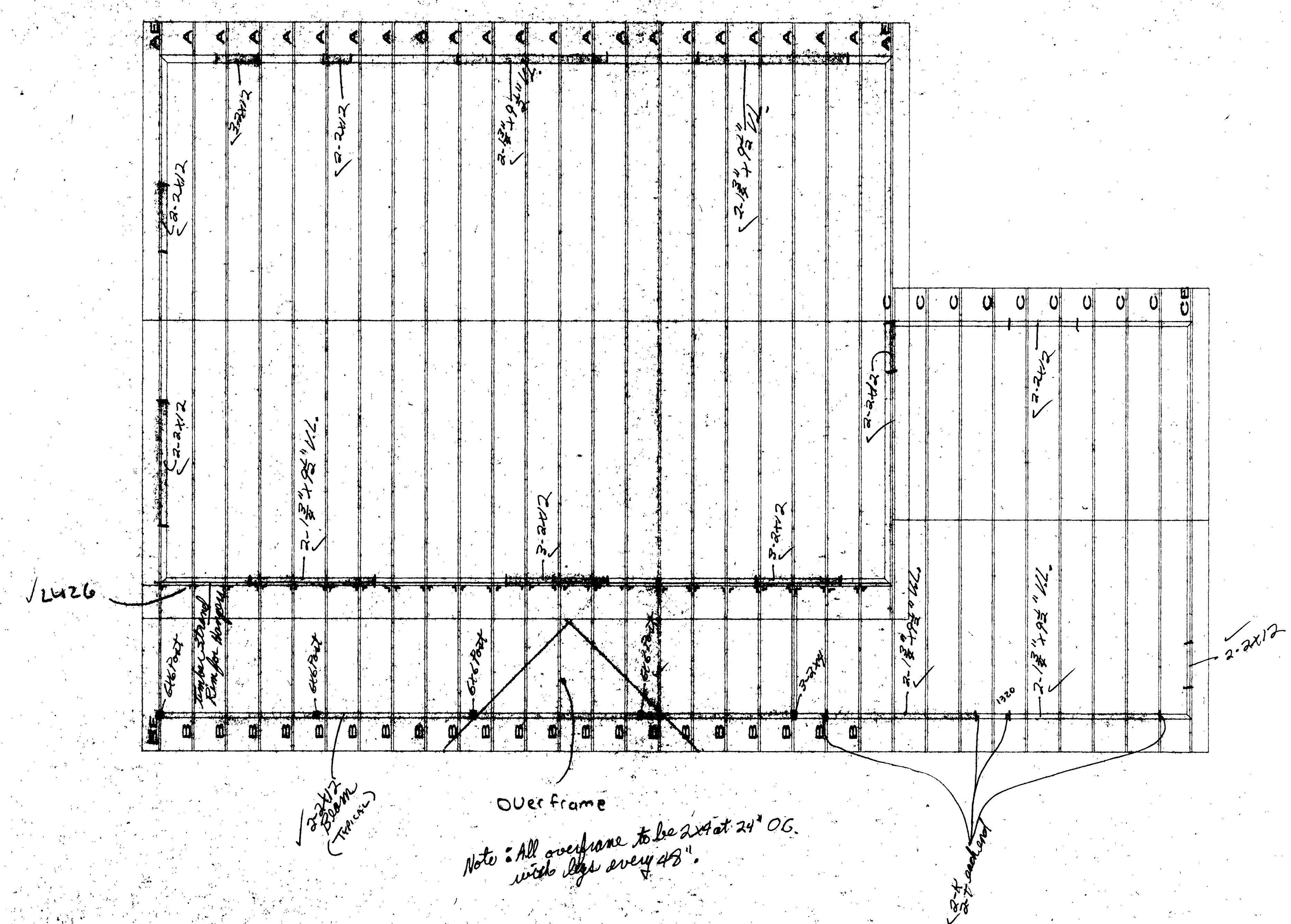
RANDAL CONSTRUCTION
PATTERSON RESIDENCE
CRISSEY FOWLER LUMBER CO.
COLORADO SPRINGS, COLORADO

SALES PRESENTATION DRAWING
No structural or dimensional check has been made with the design drawing of the building, therefore purchaser is to check and approve all dimensions, quantities, loads and details carefully. This drawing has not been checked by Boise Cascade Engineering.

SCALE: N.T.S.
DATE: 11/3/99
BY: CK
FILE: 9911611B.bcf
DWG:
SHEET: 2 / 2



REGIONAL BLDG DEPT
 JAN 07 2000
 APPROVED BY TGC



Roofline 3D Layout **RANDAL CONST. - PATTERSON** : DD
CRISSEY-1

DUE DATE :
 DSGNR/CHKR : DD / DD

TC Live	30.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
Total	45.00 psf

WO# : ROOFSAV
 SCALE : 1/4" = 1'
 Date : 11/12/1999 8:45
 DurFac-Lbr : 1.15
 DurFac-Pit : 1.15
 O.C. Spacing : 24.0
 Design Spec : UBC-97
 ST/CTg : 58 / 5

REGIONAL BLDG DEPT
 JAN 07 2000
 APPROVED BY TGC

✓RBD

Pikes Peak Regional Building Development

Permanent Plan Number 26298

Permit Number D76307

Contractor ID 10734

Temporary Plan Number 15366

Plan Type R



CITY OF COLORADO SPRINGS

April 11, 2000

Mr. Adrian Stanciu
LDC, Inc.
3520 Austin Bluffs Parkway
Colorado Springs, Colorado 80918

RE: City File Number: AR DP 00-063 & AR S 00-064F: Patterson Subdivision

Dear Adrian:

The City Planning Development Review Unit administratively approved the above-mentioned Hillside Development / Site Plan and Subdivision Plat on April 11, 2000. This approval is subject to the following conditions:

1. Development must conform completely to the approved Hillside Development / Site Plan.
2. All site grading must comply with the grading illustrated on the Hillside Development / Site Plan.

Copies of the approved Hillside Development / Site Plan and one copy of the approved Geologic Hazards Report are enclosed. Please attach one (1) copy of the approved Development Plan set to each of the two (2) sets of construction drawings submitted to the Regional Building Department in conjunction with the building permit application. A Certificate of Occupancy will not be issued for the development until all private and public improvements shown on the plan are completed or financially secured.

This Hillside Development / Site Plan approval will expire four (4) years from the approval date unless a building permit is issued for the construction of the development. If any changes to the approved site or building design become necessary prior to, or during construction, an amended plan for will need to be submitted for City Planning review and approval. Please feel free to contact me at 385-5090 if you have any questions regarding this approval.

Please submit an original mylar of the final subdivision plat to the City Land Development Administration office for final signatures, payment of fees and recording. This mylar will be routed for final departmental approvals and a signed copy will be returned to you.

Sincerely,

Larry Larsen, AICP
Development Review Planner

cc: Development Review CAB Office Files (AR DP 00-063)(AR S 00-064F)
Development Review RBD Office File (AR DP 00-063)
Dave Hornbacher - CSU Development Services
Roger Costello - Fire Prevention
Wendy Hardy - CSPD
Donna Fair - Emergency Management
City Engineering RBD Review Team
Tim Roberts - Traffic Engineering

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-2-12	1680	5.50"	2.39"
2	31-5-4	1680	5.50"	2.39"

TC 2x4 SFP C1650FL SE
 BC 2x4 SFP C1650FL SE
 WEB 2x4 HF STD
 PLATE VALUES PER ICCO RESEARCH REPORT #1607.
 Loaded for 10 BSF max-occupant BDL.

Plating spec: ANSI/AISI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 REPAIRS REQUIREMENTS shown are based ONLY
 on the truss material at each bearing.

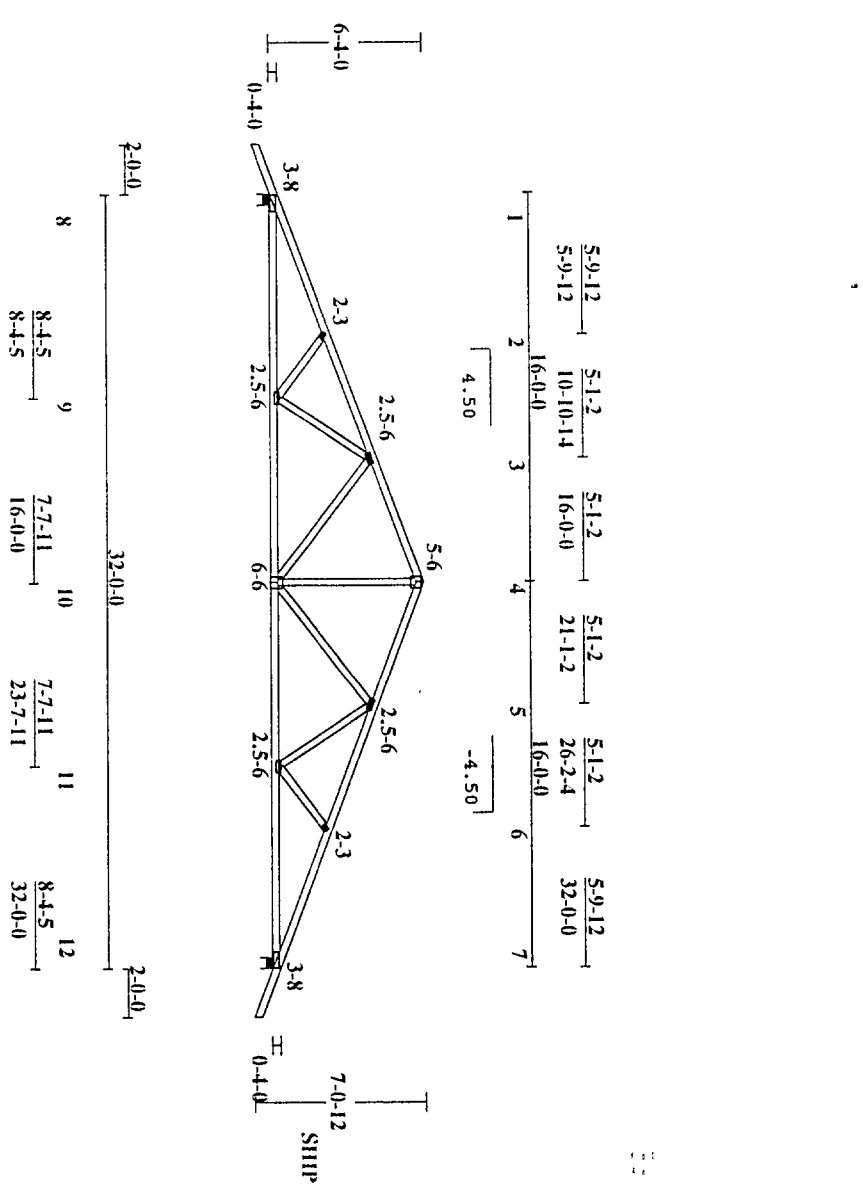
DELT REACTION(S) :
 support 1 -587#
 support 2 -587#
 This Truss is designed using the
 UBC-97 Code.
 Bldg Enclosed = Yes, Exd Zone = No
 Hurricane/Ocean Lrte = No, Exp Category = C
 Bldg Lrngth = 99.00ft, Bldg Width = 50.00ft.
 Mean roof height = 23.17ft, MEH = 85
 Classification = 4, Dead Load = 14.5 psf

BC	FORCE	AML	RBD	CSI
1	2991	.49	.13	.61
2	2466	.40	.15	.55
3	2466	.40	.15	.55
4	2991	.49	.13	.61

WEB	FORCE	CSI	WEB	FORCE	CSI
1	2-9	-393	5-10	-739	.66
2	3-9	416	5-11	416	.16
3	3-10	-739	6-11	-393	.12
4	4-10	1050			.40

MAX DEFLECTION (span) :
 L/999 MIN MAX 10-11 (LIVE)
 Lf = .27" Df = .13" Tf = -.40"

Joint	Locations	1	2	3	4	5	6
1	0-0-0	7	32	0	0	0	0
2	5-9-12	8	0	0	0	0	0
3	10-10-14	9	8	4	5	0	0
4	16-0-0	10	16	0	0	0	0
5	21-1-2	11	23	7	11	0	0
6	26-2-2	12	32	0	0	0	0



Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.

This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of T19 and A19A design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "FRICTION MAINTAIN." by Howard, QUALITY CONTROL, STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES, (QSI-880), HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES - (HHB-91) and HHB-91 STANDARD SHEET" by TPI. The Truss Plate Institute (TPI) is located at 585 D'Arbonne Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AF&PA) is located at 1740 E. Main, Okla. Ave., NW, Ste. 204, Washington, DC 20006.

117 W. VIKING AVENUE, COLO. SPRING, CO 80901
 PHONE: (719) 473-2411
 Tps. 0 Version 06.05.99

TBR: 73.3	WO: 110399B			
CHK: DD	Customer Name:			
DESIGN: DD	CRISSEY-1			
TC Live	30.0 psf	DurFacs	L=1.15	P=1.15
TC Dead	7.5 psf	Rep Max	Bad	1.15
BC Live	.0 psf	O.C. Spacing	2-0-0	0
BC Dead	7.5 psf	Design Spec	UBC-97	
TOTAL	45.0 psf	Defl Ratio:	L/240	TC: L/240

11/15/99
 Scale: 1/8" = 1'

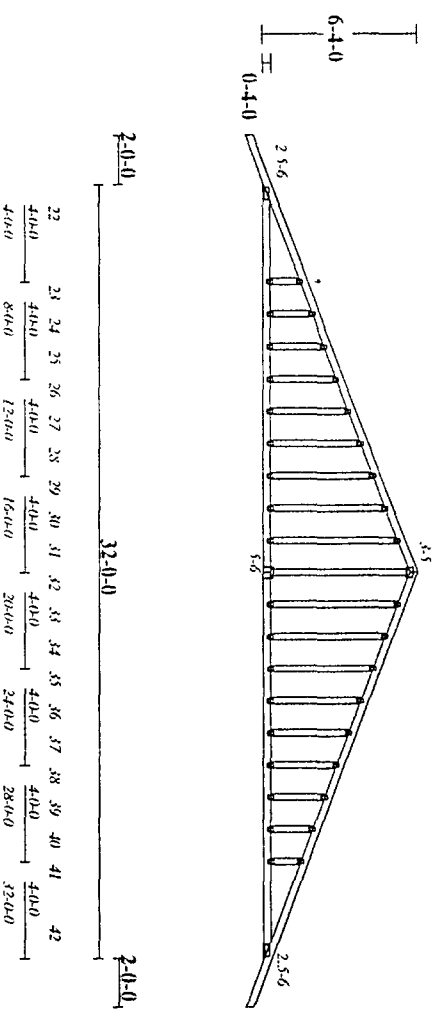
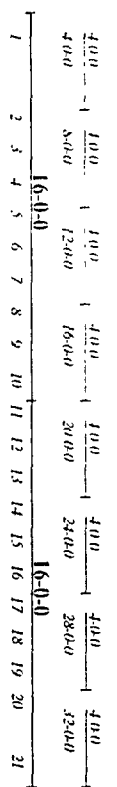
Joint Locations

1	0-0-0	22	0-0-0
2	4-0-0	23	4-0-0
3	5-4-0	24	5-4-0
4	6-8-0	25	6-8-0
5	8-0-0	26	8-0-0
6	9-4-0	27	9-4-0
7	10-8-0	28	10-8-0
8	12-0-0	29	12-0-0
9	13-4-0	30	13-4-0
10	14-8-0	31	14-8-0
11	17-0-0	32	17-0-0
12	17-4-0	33	17-4-0
13	18-8-0	34	18-8-0
14	20-0-0	35	20-0-0
15	21-4-0	36	21-4-0
16	22-8-0	37	22-8-0
17	24-0-0	38	24-0-0
18	25-4-0	39	25-4-0
19	26-8-0	40	26-8-0
20	28-0-0	41	28-0-0
21	32-0-0	42	32-0-0

TC 2x4 SFP C1650FL SE
 BC 2x4 SFP C1650FL SE
 GEL BRK 2x4 HF STD
 PLATE VALUES PER ICB3 RESEARCH REPORT #1607.

THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 BEARING REQUIREMENTS shown are based ONLY
 on the truss material at each bearing.
 Loaded for 10 PSF non-concurrent BCLL.
 < It is assumed that one face of this truss
 < is sheathed with plywood, OSB, wood board
 < siding or hardboard siding. If not,
 < additional loads must be considered on
 < non-continuous bearing gables.
 May use adequate staples for gable blocks.
 Gable blocks may require lateral bracing
 due to wind applied to the face. See
 Trusswals suggested gable bracing detail(s).
 Lateral loads in line with the chords have
 not been considered unless noted otherwise.
 These loads and their corrections are the
 responsibility of the building designer.

This truss is designed using the
 UBC-97 Code.
 Bltg Enclosed = Yes, End Zone = No
 Hurricane/Ocean Lbr = No, Exp Category = C
 Bltg Length = 99.00ft, Bltg Width = 50.00ft,
 Mean roof height = 23.17ft, MH = 85
 Classification = 4, Dead Load = 14.5 psf



TYPICAL PLATE : 2-3

Trusswals Systems Plates are 20 GA. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.

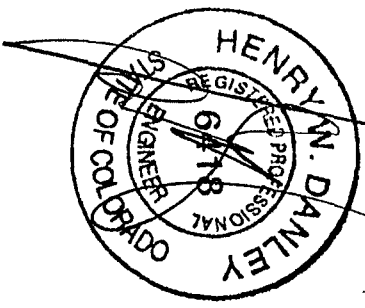
This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TP and APFA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: TRUSS MANUFACTURING, by Trusswals, QUALITY CONTROL, STANDARD FOR METAL PLATE CONNIE TIED WOOD TRUSSES - (QSC-388), HANDLING, INSTALLING AND BRACING METAL PLATE CONNIE TIED WOOD TRUSSES - (CMB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 587 D'Arden Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Commerce Ave, NW, Ste 200, Washington, DC 20036.

117 W. VERMILION AVE., COLO. SPRS, CO 80907
 PHONE: (719) 473-2411
 Tps. 0 Version 06.05.99

TBF: 97.3	MO: 110399B
Chk: DD	Customer Name:
Design: DD	#LC = 10
TC Live	30.0 psf
TC Dead	7.5 psf
BC Live	.0 psf
BC Dead	7.5 psf
TOTAL	45.0 psf
	DurFacs L=1.15 P=1.15
	Rep Mbr Bnd 1.15
	O.C.Spacing 2-0-0
	Design Spec UBC-97
	Defl Ratio: L/240 TC: L/240

11/15/99

Scale: 1/8" = 1'



RPB

JAN 0 1 2000

REQ	X-LOC	REACT	SIZE	REQ'D
1	0-1-12	604	3.50"	1.50"
2	7-11-4	356	1.50"	1.50"
TC	FORCE	ML	BD	CSI
1	-515	.00	.24	.24
2	-62	.00	.24	.24
BC	FORCE	ML	HD	CSI
1	452	.07	.15	.23
WEB	FORCE	CSI	WEB	FORCE
2-5	-497	.18	3-5	-110
				.03

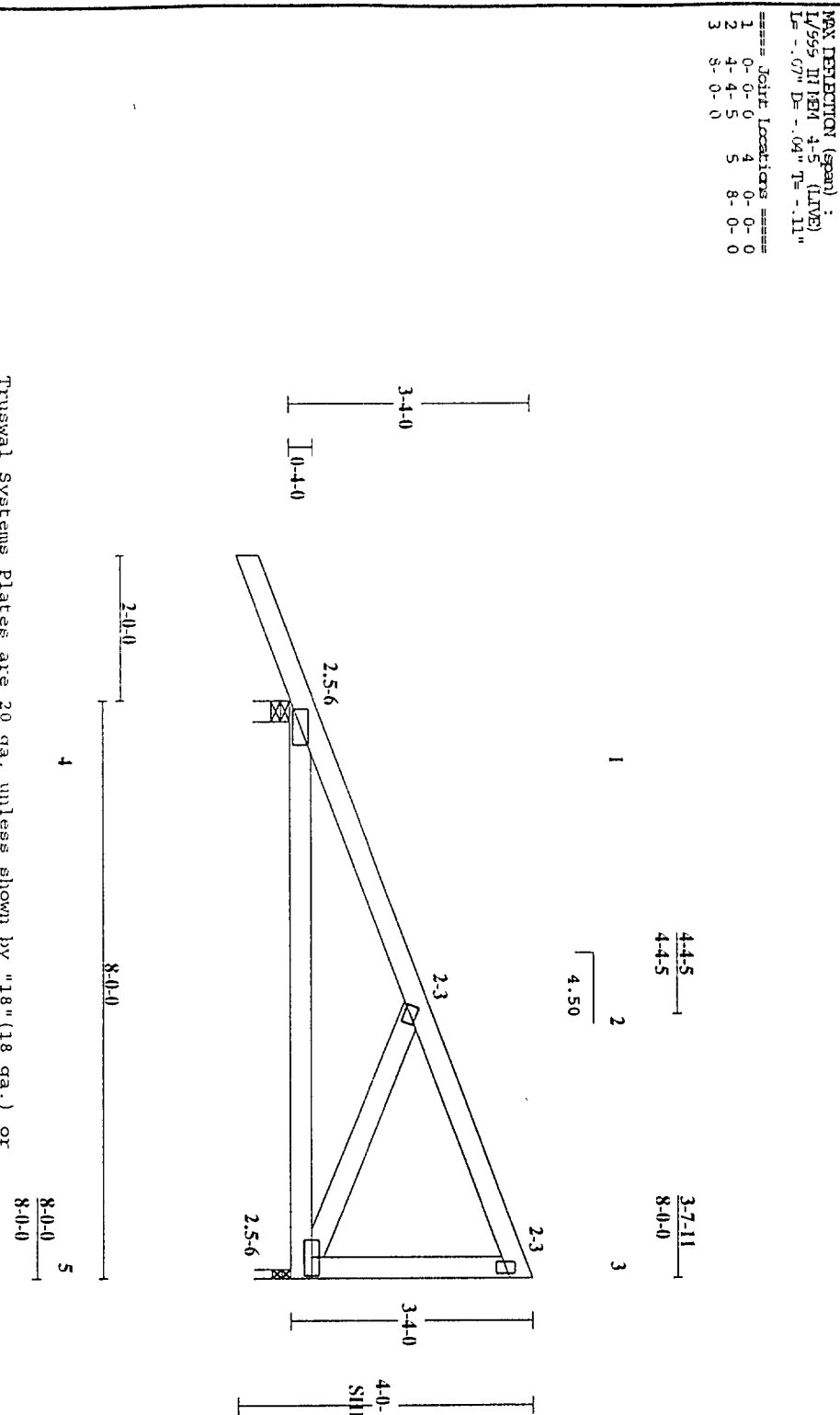
Plating spec : ANSI/AISI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 BEARING REQUIREMENTS shown are based ONLY
 on the truss material at each bearing.
 Brd verticals designed for axial loads only.
 Brd verticals that are extended above or
 below the truss profile (if any) may require
 additional design consideration (by others)
 for lateral forces due to wind or seismic
 loads on the building.

MAX DEFLECTION (span) :
 L/995 III PER 4-5 (LIVE)
 L = 11.07" D = .04" T = .11"

==== Joint Locations ====

1	0-0-0	4	0-0-0
2	4-4-5	5	8-0-0
3	8-0-0		

DEFLT	REACTION(S)
support 1	-180#
support 2	-159#
support 1	173#
support 2	173#



11/15/99
 Scale: 1/32" = 1'

HENRY W. DANLEY
 REGISTERED PROFESSIONAL
 ENGINEER
 STATE OF COLORADO
 No. 6418

CRISSEY

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.

This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AIA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "BRACING MANUAL", by Trussal, QUALITY CONTROL, STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QST-88), "HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 581 Drexel Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AF&PA) is located at 1290 Connecticut Ave, NW, Suite 200, Washington, DC 20036.

CRISSEY
 FOWLER

117 W. ALHAMBRA AVE. (COR. G. STERN) (E. 80th St)
 PHONE: (7719) 473-2411
 Tps. 0 Version 06.05.99

TBF:	18.0	WO: 110399B
CHK: DD		Customer Name:
Design: DD	#LC = 10	CRISSEY-1
TC Live	30.0 psf	DirFacs L=1.15 P=1.15
TC Dead	7.5 psf	Rep Mbr Bad 1.15
BC Live	.0 psf	O.C.Spacing 2-0-0
BC Dead	7.5 psf	Design Spec UBC-97
TOTAL	45.0 psf	Defl Ratio: 1/240 TC: 1/240

BRG	X-LOC	REACT	SIZE	REQ'D	REQ'D
1	0-11.2	604	3.50"	1.50"	1.50"
2	7-11-4	356	1.50"	1.50"	1.50"

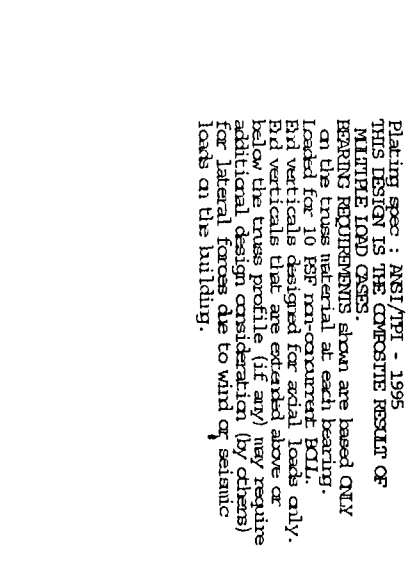
TC FORCE 2x4 BD CSI
 1 -515 .06 .24 .24
 2 -62 .00 .24 .24

BC FORCE 2x4 HD CSI
 1 .07 .15 .23

MBR FORCE CSI MB FORCE CSI
 2-5 -497 .18 3-5 -110 .03

MAX DEFLECTION (span) :
 L/595 IF HBH 4-5 (LIVE)
 L_s = .07" D = .04" T_s = .11"

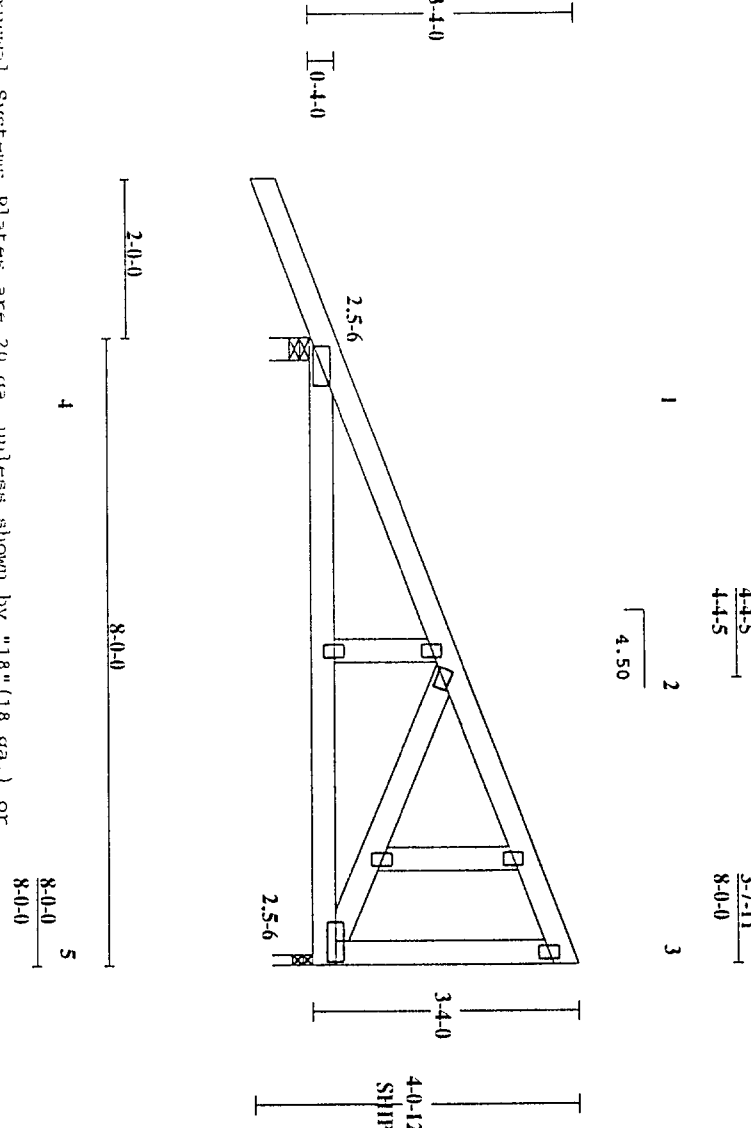
==== Joint Locations =====



Plating spec : ANSI/APA - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 BEARING REQUIREMENTS shown are based ONLY
 on the truss material at each bearing.
 Loaded for 10 RRF non-compact BOLD.
 Bldg verticals designed for axial loads only.
 Bldg verticals that are extended above or
 below the truss profile (if any) may require
 additional design consideration (by others)
 for lateral forces due to wind or seismic
 loads on the building.

UPLIFT REACTION(S) :
 support 1 -180#
 support 2 -153#
 HORIZONTAL REACTION(S) :
 support 1 173#
 support 2 173#
 This truss is designed using the
 TRC-97 Code.
 Bldg Enclosed = Yes, Bld Zone = No
 Hurricane/Ocean Line = No Expo Category = C
 Bldg Height = 99.00ft, Bldg Width = 50.00ft,
 Mean roof height = 21.51ft, WEFH = 85'
 Classification = 4, Dead Load = 14.5 psf

TYPICAL PLATE : 2-3



TBF: 20.7
 Chk: DD
 Degr: DD #LC = 10

TC Live 30.0 psf
 TC Dead 7.5 psf
 BC Live .0 psf
 BC Dead 7.5 psf

WO: 110399B
 Customer Name: CRISSEY-1
 DurFace L=1.15 P=1.15
 Rep Mbr Bnd 1.15
 O.C. Spacing 2-0-0
 Design Spec UBC-97
 Defl Ratio: L/240 TC: 1/240

11/15/99
 Scale: 13/32" = 1"

11" W. L. FOWLER, VICE PRESIDENT, CRISSEY FOWLER
 PHONE: (719) 473-2411
 Truss-0 Version 06.05.99

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and APA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for moisture content of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "FRUSTRATION-MANUAL", by Trusswood, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QIB-97) and "HIB-91 SUPPLEMENTARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 8831 D'Oroville Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1290 Connecticut Ave. NW, Ste 200, Washington, DC 20006.

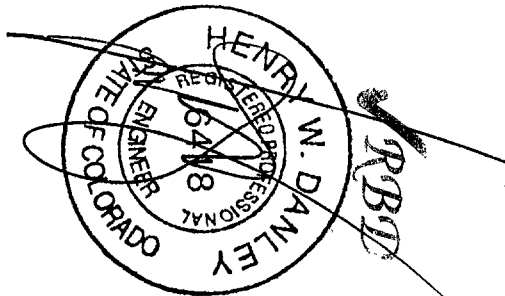
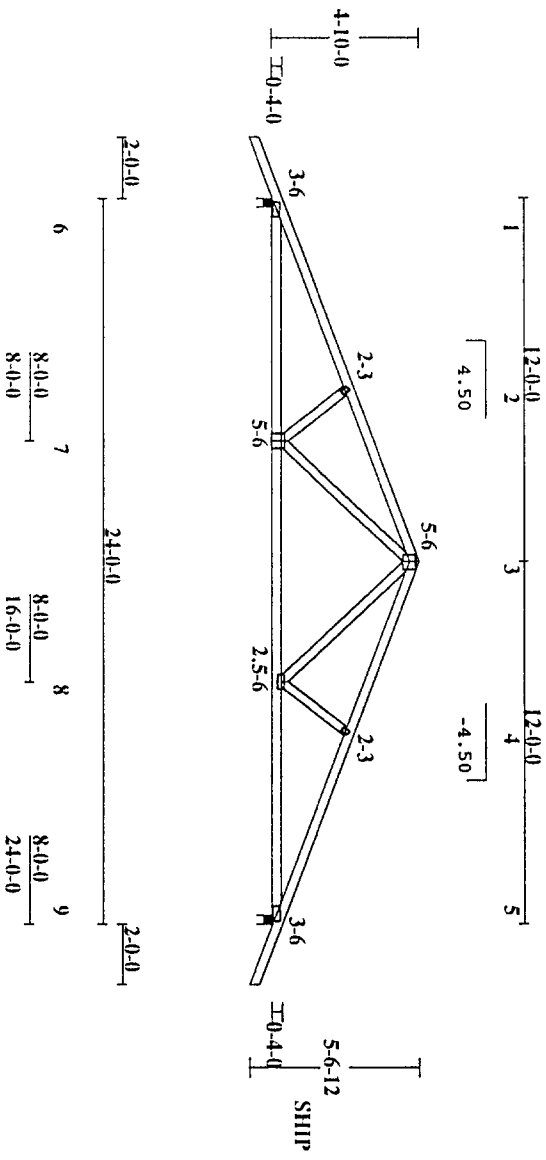
HENRY W. DANLEY
 REGISTERED PROFESSIONAL ENGINEER
 STATE OF COLORADO
 No. 418

BEG	X-LOC	REACT	SIZE	REQ'D	TC	2x4	SFP	CLASS	REQ'D
1	0-1-12	1320	3.50"	1.88"	BC	2x4	SFP	CLASS	REQ'D
2	23-10-4	1320	3.50"	1.88"	BC	2x4	SFP	CLASS	REQ'D
PLATE VALUES PER ICBO RESEARCH REPORT #1607. Loaded for 10 PSP INT-COMPACTION EQL.									

TC	2x4	SFP	CLASS	REQ'D
1	2x4	SFP	CLASS	REQ'D
2	2x4	SFP	CLASS	REQ'D
3	2x4	SFP	CLASS	REQ'D
4	2x4	SFP	CLASS	REQ'D

BEF	FORCE	CSI	WEB	FORCE	CSI
1	0-	0-	0-	0-	0-
2	6-	4-	5-	8-	0-
3	12-	0-	0-	16-	0-
4	17-	7-	11-	24-	0-
5	24-	0-	0-		

MAX DEFLECTION (span)	L/595 IN BEH	7-8 (LIVE)
$D = -.16"$	$D = -.08"$	$T = -.24"$



WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of HP and ALPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connection plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: TRUSS MANUFACTURE, by Trussal, QUALITY CONTROL, STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES - (QST1-88), HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES - (HBP-91) and HBP-91 SUPPLEMENTARY NOTE: F by Trussal. The Truss Plate Institute (TPI) is located at 881 D'Amico Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave., NW, No 200, Washington, DC 20036.

117 W. LAUREL AVE., CHLO. ST. (7030291)
 PHONE: (719) 473-2411
 Type: 0 Version 06.05.99

Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.

11/15/99
 Scale: 5/32" = 1'

TBF:	49.3	MO:	110399B
CHK:	DD	Customer Name:	CRISSEY-1
Degrat:	DD	#IC =	10
TC Live	30.0 psf	DurFace	L=1.15 P=1.15
TC Dead	7.5 psf	Rep Mbr	Bnd 1.15
BC Live	.0 psf	O.C.Spacing	2- 0- 0
BC Dead	7.5 psf	Design Spec	UBC-97
TOTAL	45.0 psf	Defl Ratio:	L/240 TC: L/240

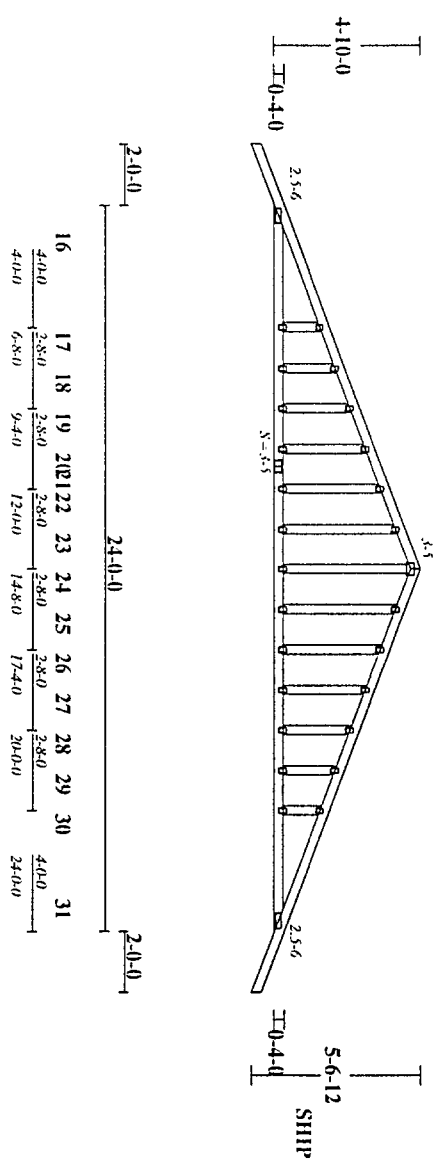
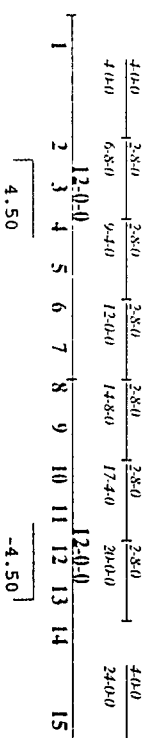
Joint Locations

1	6-0-0	17	4-0-0
2	4-0-0	18	5-4-0
3	5-4-0	19	6-8-0
4	6-8-0	20	8-0-0
5	8-0-0	21	8-6-14
6	9-4-0	22	9-4-0
7	10-8-0	23	10-8-0
8	12-0-0	24	12-0-0
9	13-4-0	25	13-4-0
10	14-8-0	26	14-8-0
11	16-0-0	27	16-0-0
12	17-4-0	28	17-4-0
13	18-8-0	29	18-8-0
14	20-0-0	30	20-0-0
15	24-0-0	31	24-0-0

TC 2x4 SPF CL650FL SE
 BC 2x4 SPF CL650FL SE
 GFL BRK 2x4 HF STD
 PLATE VALUES PER ICBQ RESEARCH REPORT #1607.

THIS DESIGN IS THE COMPOSITE RESULT OF MEMBER LOAD CASES. BEARING REQUIREMENTS shown are based ONLY on the truss material at each bearing. Loaded for 10 PSF non-snow load. PULL-OUT IS ASSUMED THAT ONE FACE OF THIS TRUSS < IS SCHEDULED WITH PLYWOOD, OSB, WOOD BOARD < SIDING OR HARDBOARD SIDING. IF NOT, < ADDITIONAL LOADS MUST BE CONSIDERED ON < NON-CONTINUOUS BEARING GABLES. May use adequate staples for gable blocks. Gable blocks may require lateral bracing due to wind applied to the face. See Trussale suggested gable bracing detail (s). Lateral loads in line with the gables have not been considered unless noted otherwise. These loads and their corrections are the responsibility of the building designer.

This truss is designed using the UBC-97 Code. Bldy Enclosed = Yes, Bld Zone = No Hurricane/Ocean Lave = No Expo Category = C Bldy Length = 99.00ft, Bldy Width = 50.00ft, Mean roof height = 22.42ft, MH = 85 Classification = 4, Dead Load = 14.5 psf



TYPICAL PLATE: 2-3

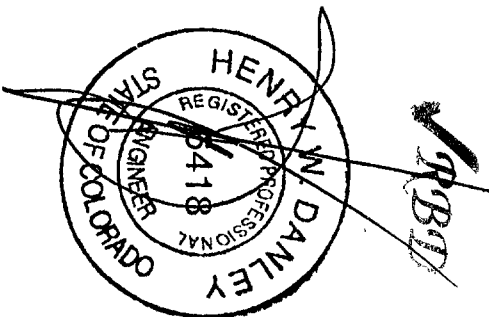
Trussal Systems Plates are 20 ga. unless shown by "18" (18 ga.) or "H" (16 ga.), positioned per Joint Report. Circled plates and false frame plates are positioned as shown above.

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.

This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AIA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSS MANUFACTURING QUALITY CONTROL STANDARDS FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QST-880), "HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (HBR-91) and "HUB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 5811 Camino Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave., NW, Ste 300, Washington, DC 20006.

CRISSEY FOWLER
 117 W. WINDING AVE., COLO SPRING, CO 80901
 PHONE: (719) 473-2411
 TFS 0.0 Version 06.05.99

TC Live	30.0 psf	DurFace	L=1.15	P=1.15
TC Dead	7.5 psf	Rep Mbr	Bnd	1.15
BC Live	.0 psf	O.C.Spacing	2-0-0	0
BC Dead	7.5 psf	Design Spec	UBC-97	
TOTAL	45.0 psf	Defl Ratio:	L/240	TC: L/240



11/15/99

Scale: 5/32" = 1'

TFS: 66.0
 Chk: DD
 Designer: DD #LIC = 10
 NO: 110399B
 Customer Name: CRISSEY-1